

STATE	ROUTE	FEDERAL AID PROJECT	ROUTE	STATE PROJECT	SHEET NO.
VA.	—	STP-5104 (326)	—	U000-104-365, B620, C501, P101	1
Federal Structure No.		20073	FHWA Construction and Scour Code:		X271-SN
Federal Stewardship and Oversight Code:				UPC No. 118295	

**DESIGN EXCEPTION(S):**  
None

**GENERAL NOTES:**

Width: 3'-0" shoulder, 22'-0" roadway, 28'-0" face-to-face of curbs, 30'-4" overall width

Span layout: 173'-0" steel girder, single span

Capacity: HL-93 loading.

**Specifications:**

Construction: Virginia Department of Transportation Road and Bridge Specifications, 2020; and SP407-000500 (Special Provision for Superstructure Erection Stability).

Design: AASHTO LRFD Bridge Design Specifications 8th Edition, 2017; and VDOT Modifications.

Standards: Virginia Department of Transportation Road and Bridge Standards, 2016; including all current revisions.

These plans are incomplete unless accompanied by the Supplemental Specifications and Special Provisions included in the contract documents.

Design loading includes 20 psf allowance for construction tolerances and construction methods.

Design loading includes 15 psf allowance for future wearing surface.

All structural steel shall be ASTM A709 Grade 50W and shall be unpainted, except the elements specified on sheet XX.

Finish paint color shall be brown, Federal Standard Color No. 595-20059.

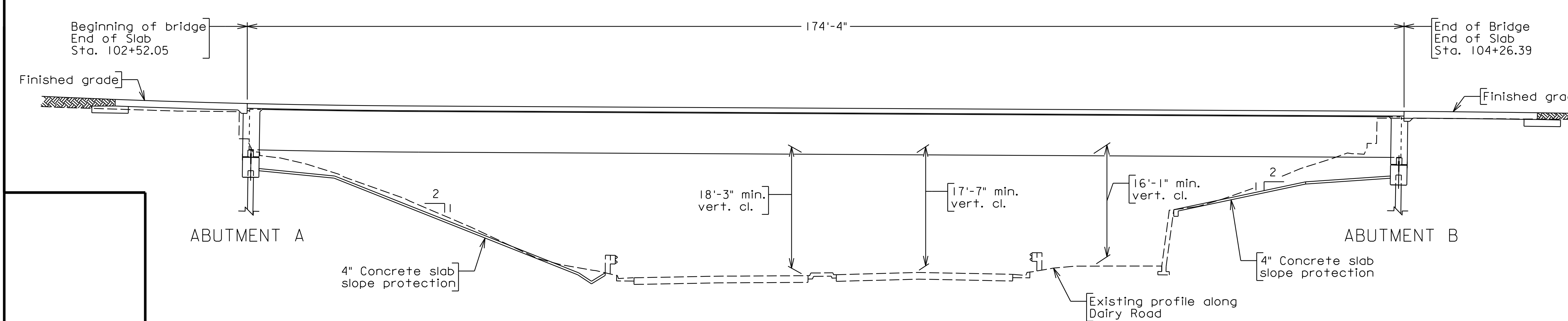
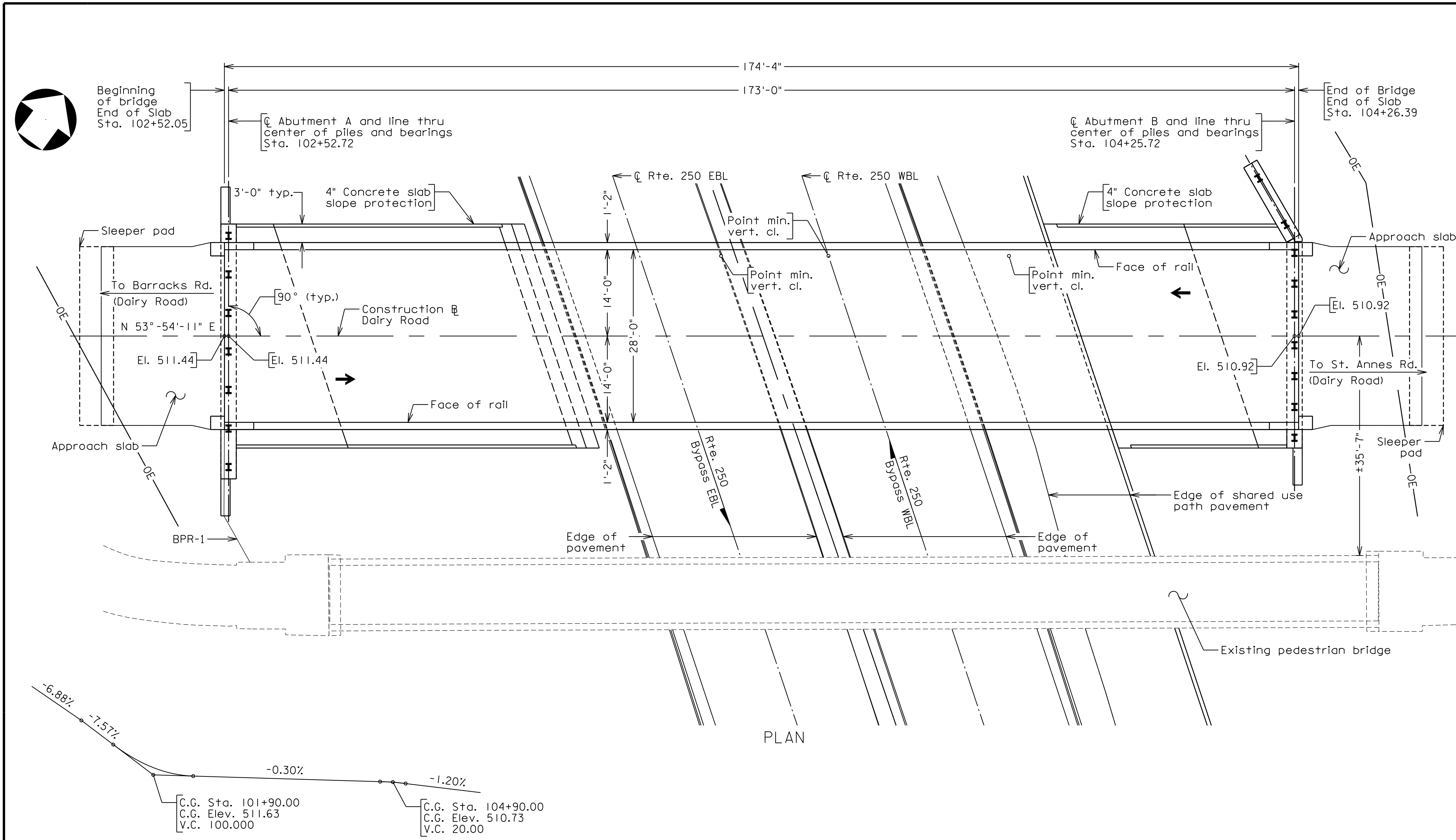
Concrete in superstructure, rails, terminal walls, and integral backwalls shall be Low Shrinkage Class A4 Modified in accordance with Section 217.12(a); in abutments and wingwalls, Class A3.

All reinforcing steel shall be deformed and shall conform to ASTM A615 Grade 60 except for steels noted as Corrosion Resistant Reinforcing (CRR) which shall conform to Section 223 of the Specifications. All reinforcing bar dimensions on the detailed drawings are to centers of bars except where otherwise noted and are subject to fabrication and construction tolerances.

The Class(es) of CRR Steel(s) required on this project is/are noted on plan sheets and in the reinforcing steel schedule. Class III may be substituted for Class I.

All H-piles shall be ASTM A709 Grade 50 steel. All piles shall be driven to the required nominal axial resistance. For axial resistance requirements, see the Pile Data Table on Sheet 10.

Bridge No. of existing bridge is 104-8000. Existing bridge not shown for clarity.



**CITY OF CHARLOTTESVILLE  
DEPARTMENT OF PUBLIC WORKS**

**PROPOSED BRIDGE ON  
DAIRY ROAD  
OVER ROUTE 250 BYPASS  
ALBEMARLE CO. - 0.4 MI. E. RUGBY RD.**

PROJ. U000-104-365, B620, C501, P101

KIMLEY-HORN & ASSOC.  
RICHMOND, VA  
STRUCTURAL ENGINEER

PLANS BY: Kimley-Horn & Associates, Inc.  
COORDINATED: Jonathan H. Oliver  
SUPERVISED: Daniel G. Davis  
DESIGNED: Levon C. Hoomes  
DRAWN: Donald A. Matthews  
CHECKED: Grant S. Curtiss

**90% PLANS**  
**THESE PLANS NOT TO BE USED FOR CONSTRUCTION**

DEVELOPED SECTION ALONG CONSTRUCTION B

Scale: 1" = 10'-0"

No.	Description	Date
REVISIONS		
For Table of Revisions, see Sheet 2.		

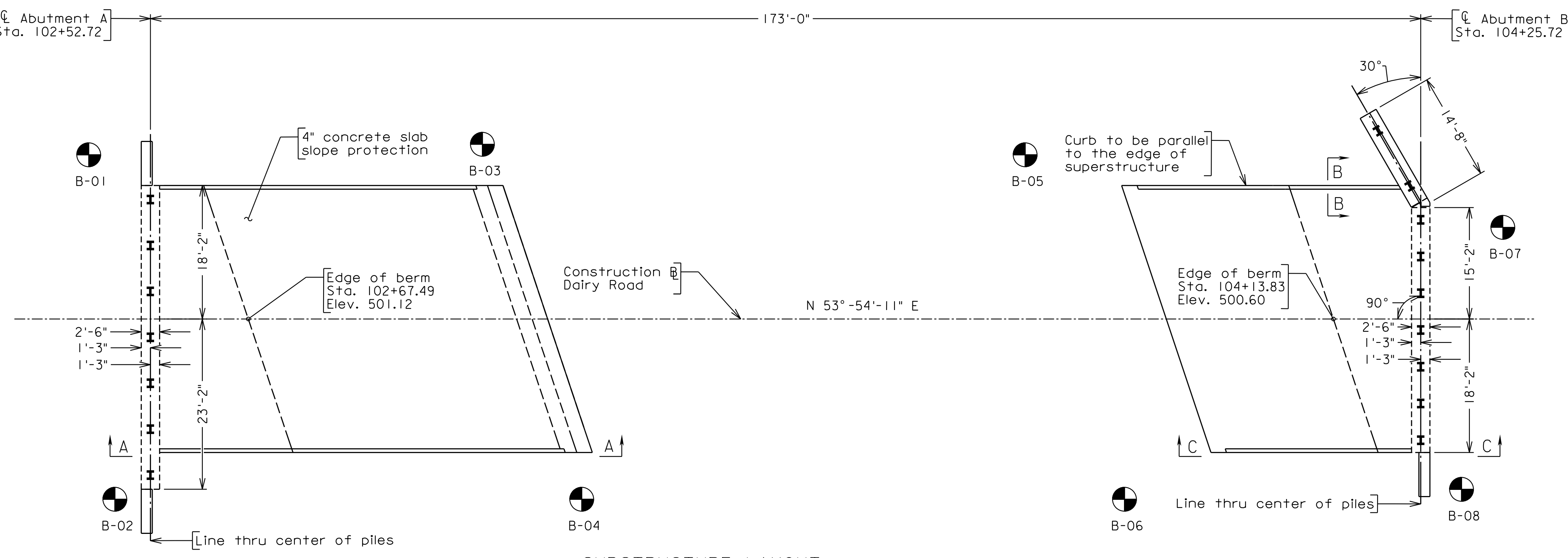
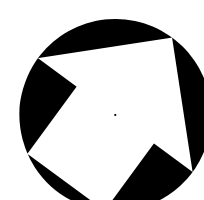
Recommended for Approval: \_\_\_\_\_ Date \_\_\_\_\_  
City Engineer

Approved: \_\_\_\_\_ Date \_\_\_\_\_  
Chief Engineer

Date: January 2026      © 2026, Commonwealth of Virginia      Sheet 1 of 30



STATE	FEDERAL AID	STATE	SHEET NO.
VA.	PROJECT STP-5104 (326)	ROUTE U000-104-365, B620, C501, P101	PROJECT

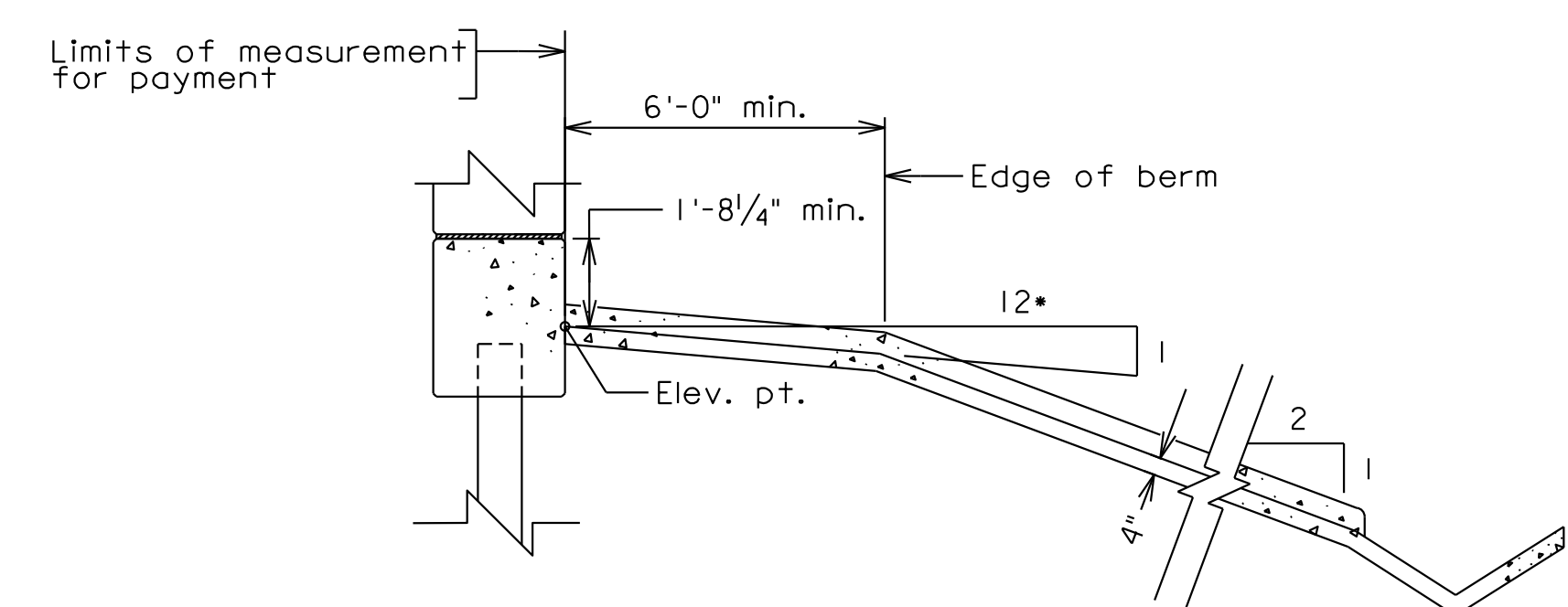


Notes:  
 This layout is to be used only for the purpose of locating fill slopes and footings. For details not shown, see abutment details.  
 Materials in the abutment select backfill zone shall be Select Material Type 1, minimum CBR 30, and shall be compacted in accordance with Sections 303 and 305 of the VDOT Road and Bridge Specifications. 21A or 21B may be substituted for Select Material Type 1, minimum CBR 30, at no additional cost to the Department.

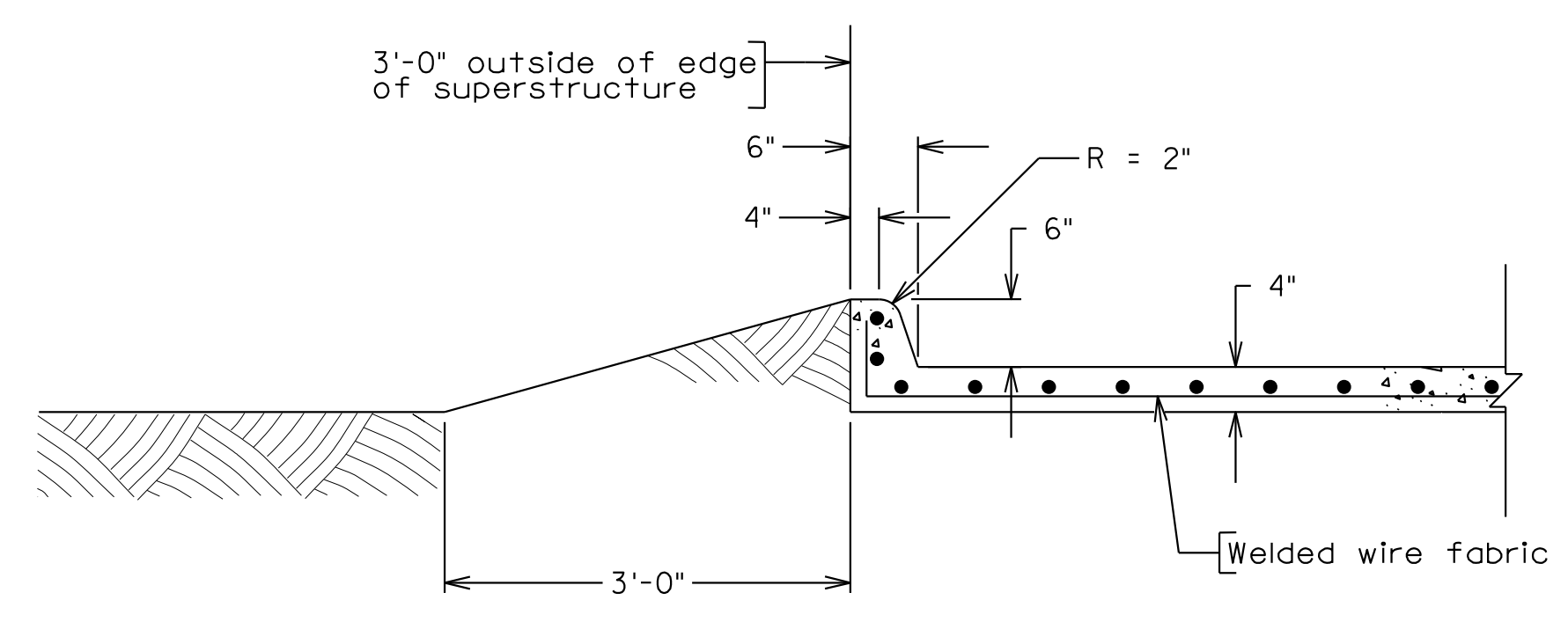
☉ = Boring locations, for details see sheets XX thru XX.

Location	Elev. pt.
Abutment A	501.62
Abutment B	501.10

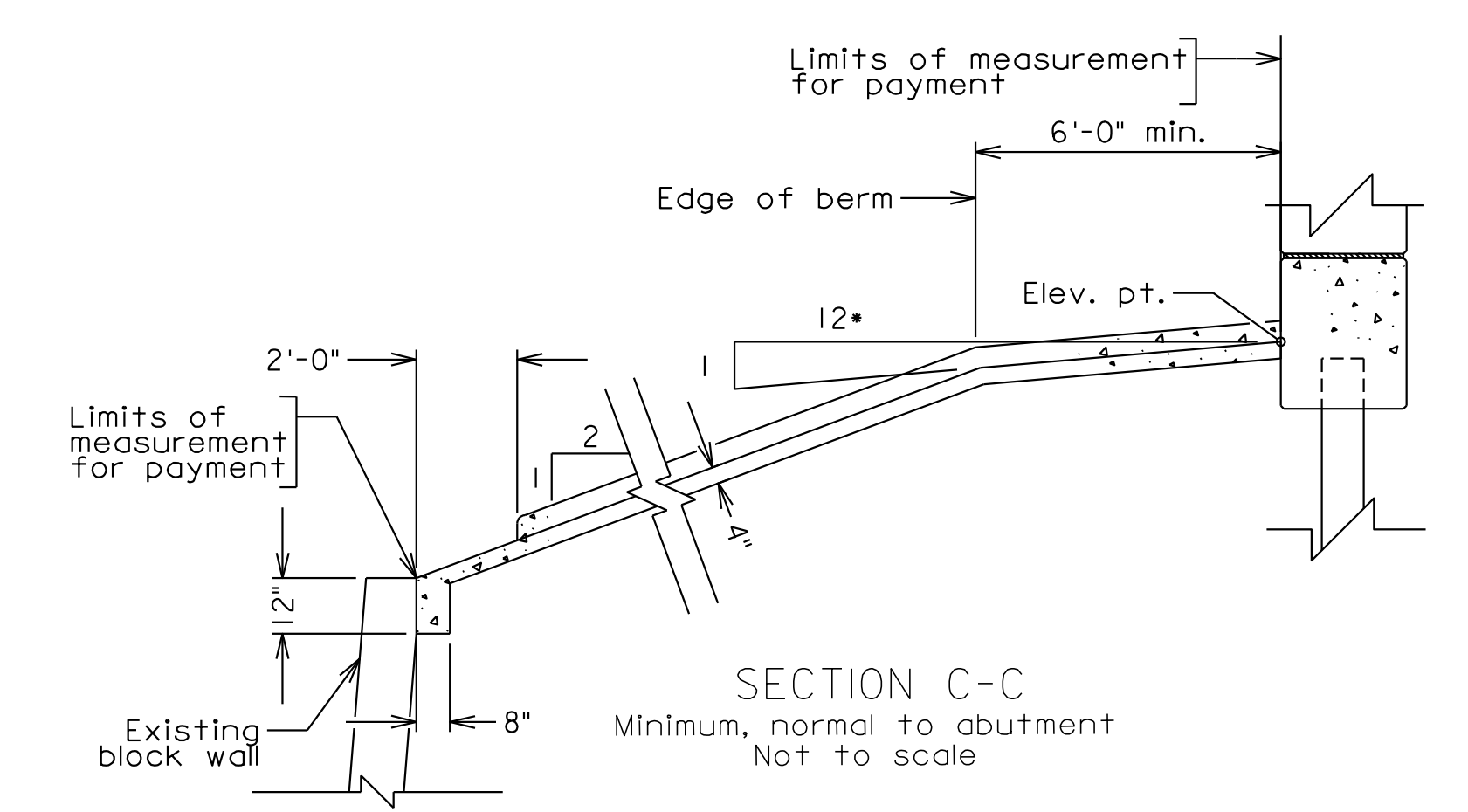
SUBSTRUCTURE LAYOUT



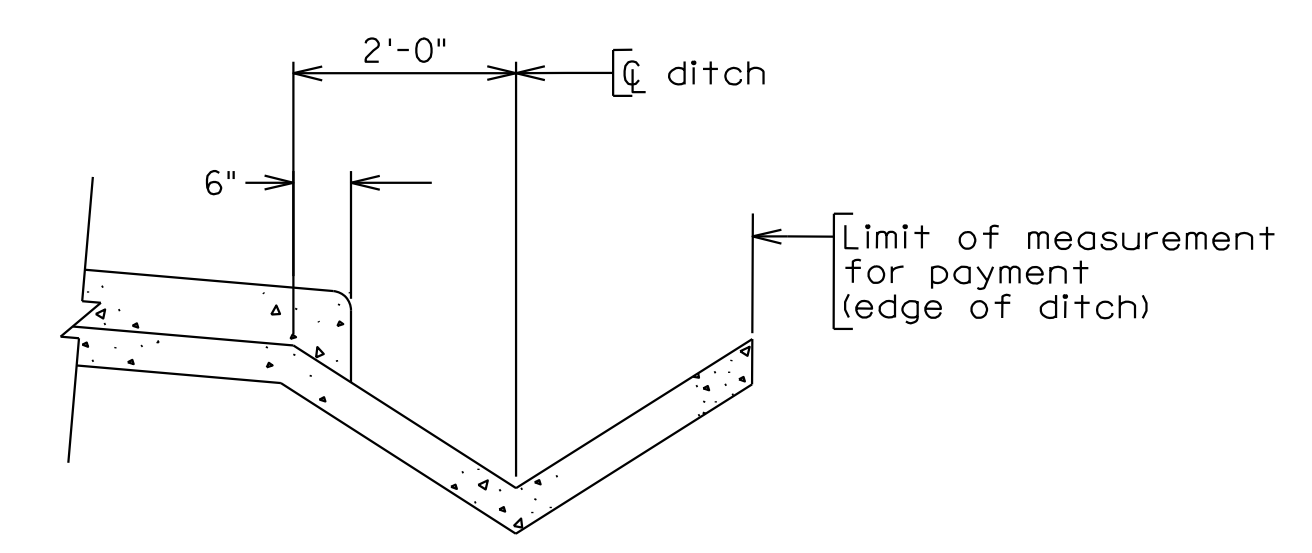
SECTION A-A  
 Minimum, normal to abutment  
 Not to scale



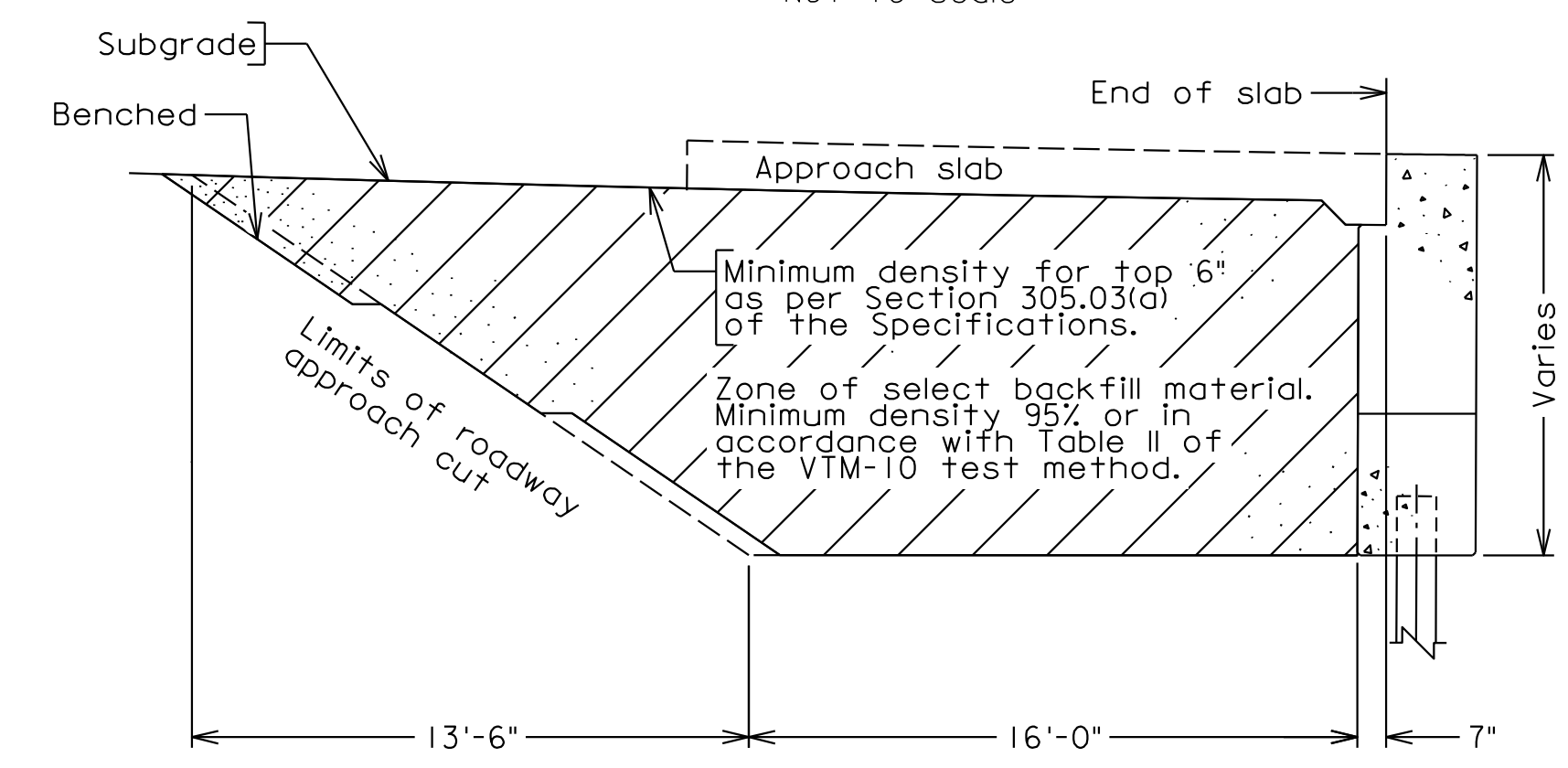
SECTION B-B  
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 Not to scale



SECTION C-C  
 Minimum, normal to abutment  
 Not to scale



PART SECTION A-A  
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 Not to scale



SECTION THROUGH ABUTMENT  
 Abutment drainage not shown  
 Not to scale

To be paid for as structure excavation

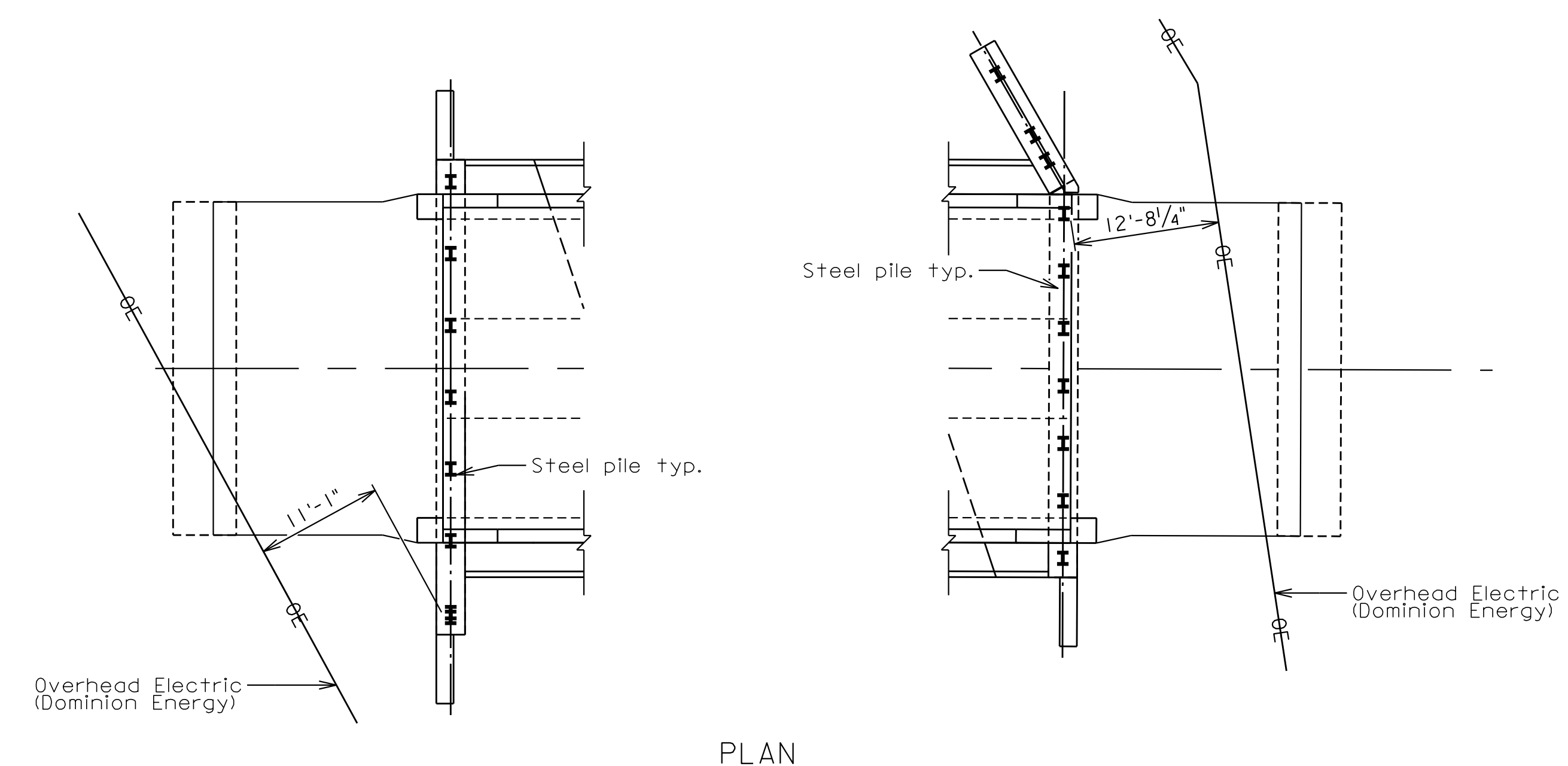
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		CITY OF CHARLOTTESVILLE	
		SUBSTRUCTURE LAYOUT, AND SLOPE PROTECTION	
No.	Description	Date	Sheet No.
	Revisions	Designed: GSC Drawn: DAM Checked: LCH	Date: January 2026 Plan No. 3 of 30

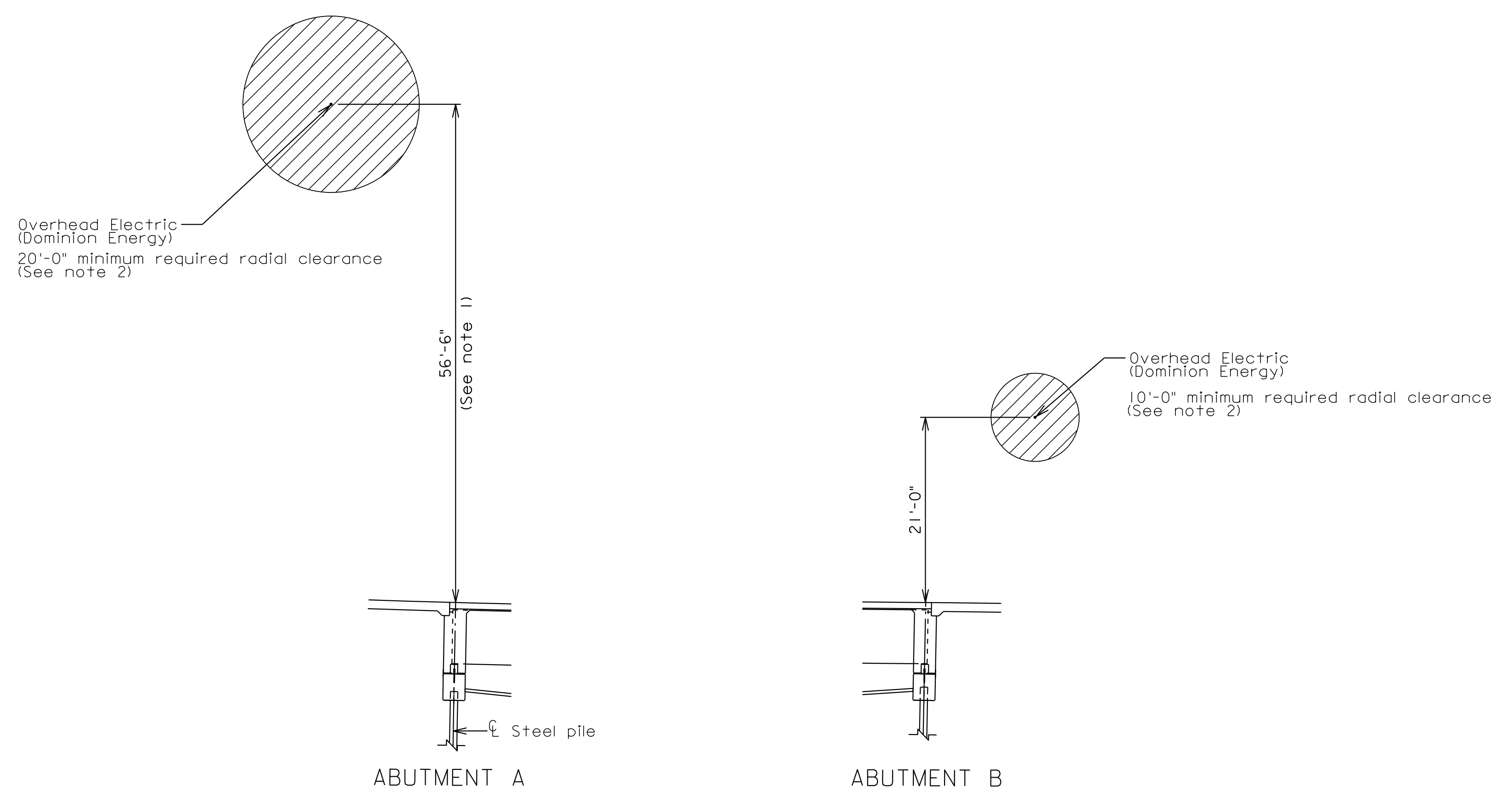
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 RICHMOND, VA  
 STRUCTURAL ENGINEER

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 THESE PLANS NOT TO BE USED  
 FOR CONSTRUCTION

STATE	FEDERAL AID		STATE		SHEET
ROUTE	PROJECT		ROUTE	PROJECT	NO.
VA.	STP-5104 (326)		—	U000-104-365, B620, C501, P101	—



PLAN



DEVELOPED SECTION ALONG CONSTRUCTION B

Notes:

1. Dimension verification: The dimensions, elevations, and intersecting angles shown are based on survey of the existing utilities and may not represent as-built conditions. It is the contractor's responsibility to verify this data before beginning construction and notify the engineer of any discrepancies.
2. Minimum required clearances provided in these plans are given based on 29 CFR Section 1926.1408, Table A and voltage information provided by the utility owner (Dominion Energy).
3. Contractor shall coordinate with Dominion Power for clearance requirements prior to beginning pile driving operations.

Note:

This is a schematic only. It is not a working drawing. The contractor shall submit complete details of the proposed erection methods and equipment in accordance with the specifications.

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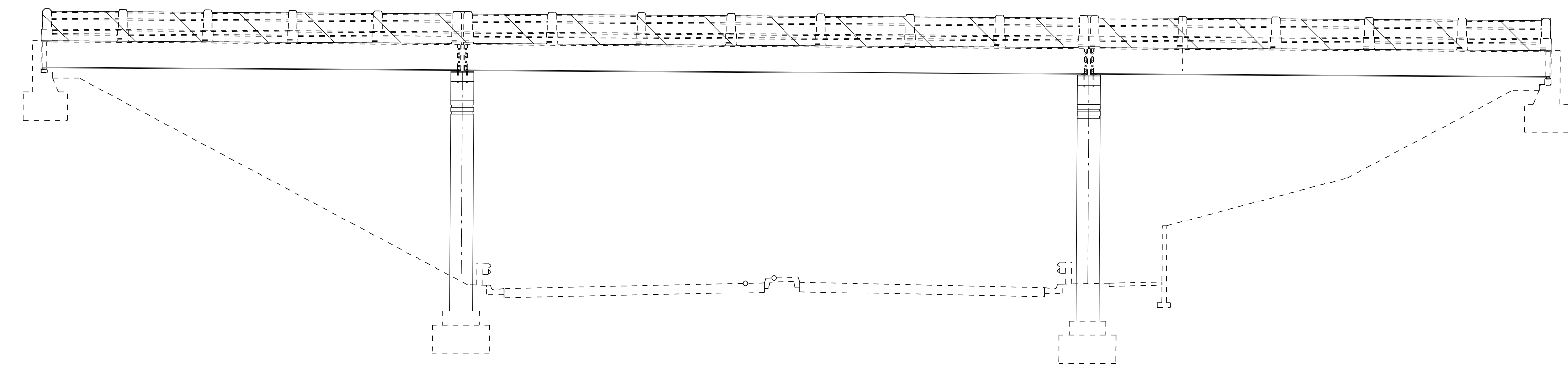
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CITY OF CHARLOTTESVILLE					
SEQUENCE OF CONSTRUCTION OVERHEAD UTILITY LOCATIONS					
No.	Description	Date	Designed: GSC	Date	Plan No.
			Drawn: DAM	January	
			Checked: LCH	2026	
Revisions					Sheet No.
					4 of 30

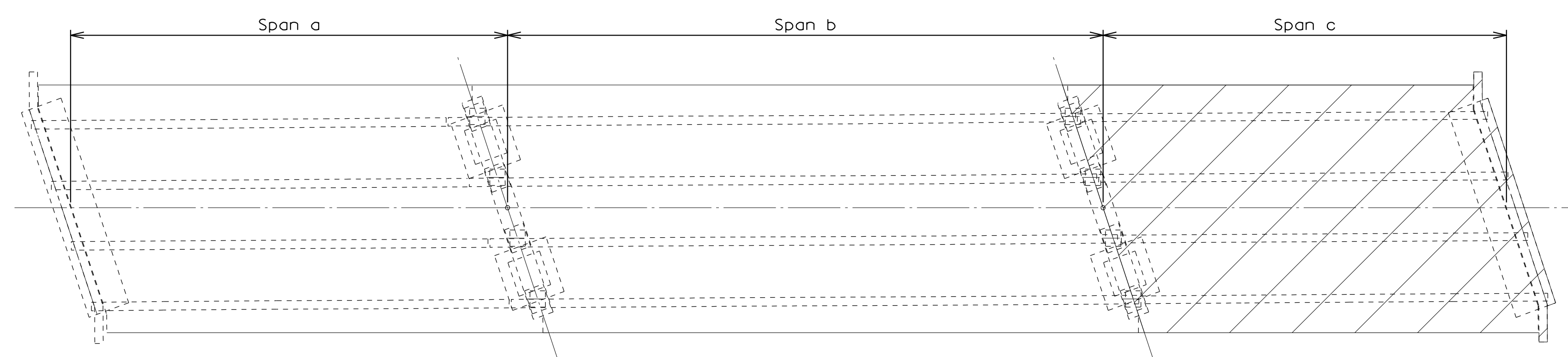
STATE	FEDERAL AID		STATE		SHEET
ROUTE	PROJECT		PROJECT		NO.
VA.	STP-5104 (326)		U000-104-365, B620, C501, P101		--



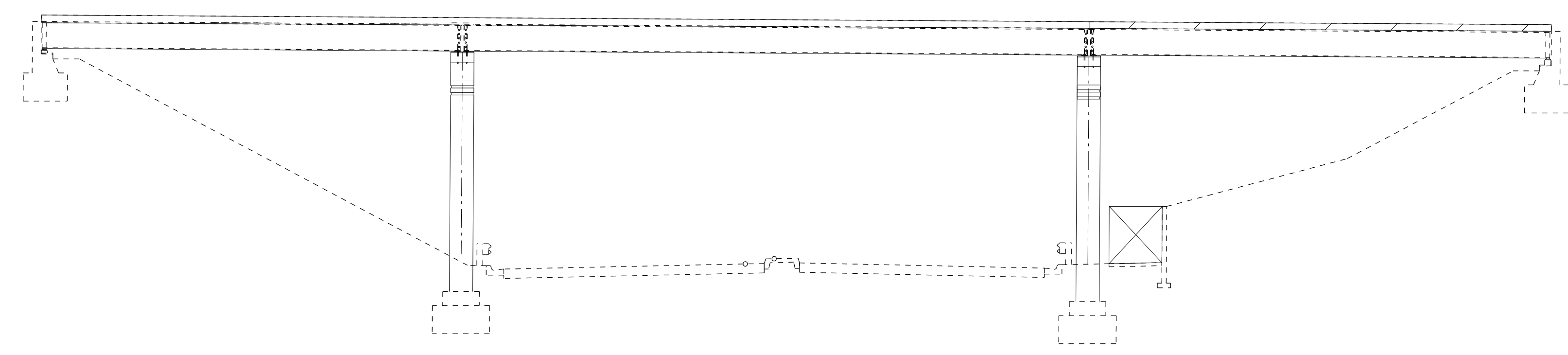
PLAN



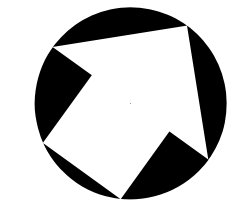
ELEVATION  
PHASE I



PLAN



ELEVATION  
PHASE 2a



- PHASE 1:**
1. Close Dairy Road bridge to traffic
  2. Remove railing and curb for both sides of bridge.

- PHASE 2a:**
1. Close Shared path.
  2. Remove slab on Span c.

Note: Contractor to check stability of Pier 2 prior to the removal of slab on Span c.

Note:  
This is a schematic only. It is not a working drawing. The contractor shall submit complete details of the proposed erection methods and equipment in accordance with the specifications.

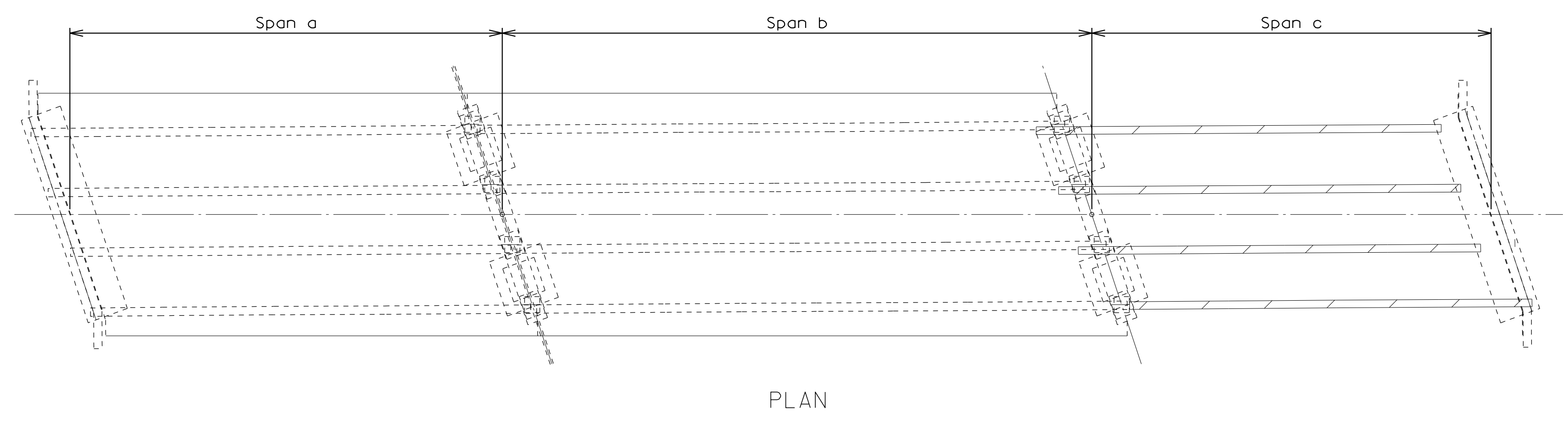
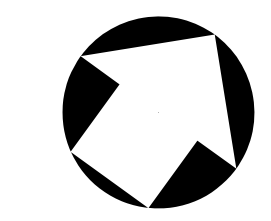
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CITY OF CHARLOTTESVILLE					
SEQUENCE OF CONSTRUCTION DEMOLITION PLAN 1 OF 4					
No.	Description	Date	Designed: KMR	Date	Plan No.
			Drawn: DAM	January	
			Checked: GSC	2026	
Revisions					Sheet No.
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STATE	FEDERAL AID		STATE		SHEET
ROUTE	PROJECT		ROUTE	PROJECT	NO.
VA.	STP-5104 (326)		—	U000-104-365, B620, C501, P101	—

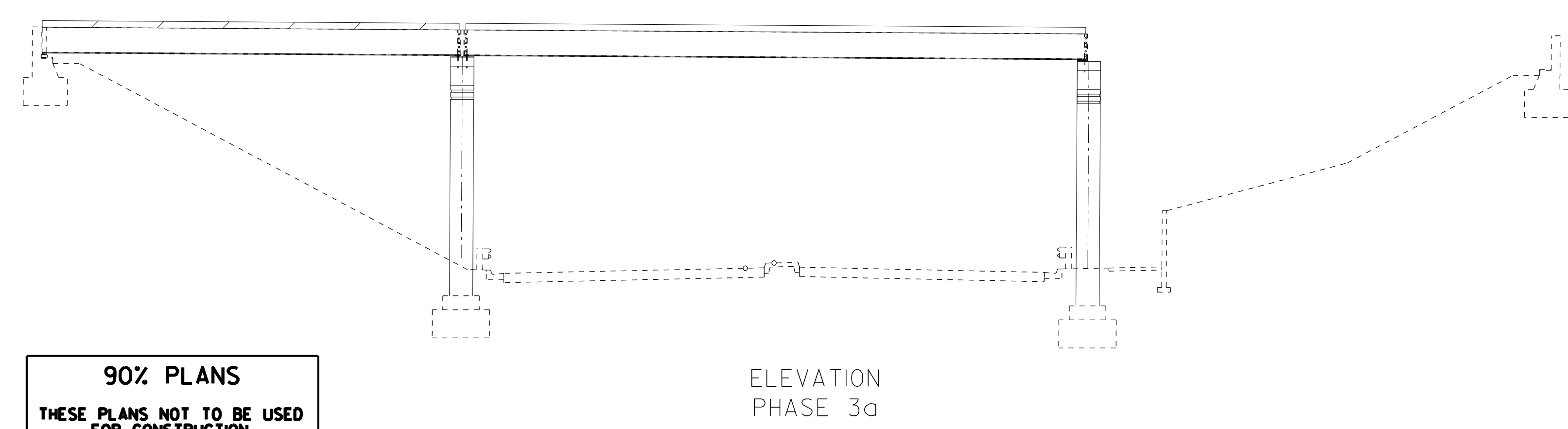
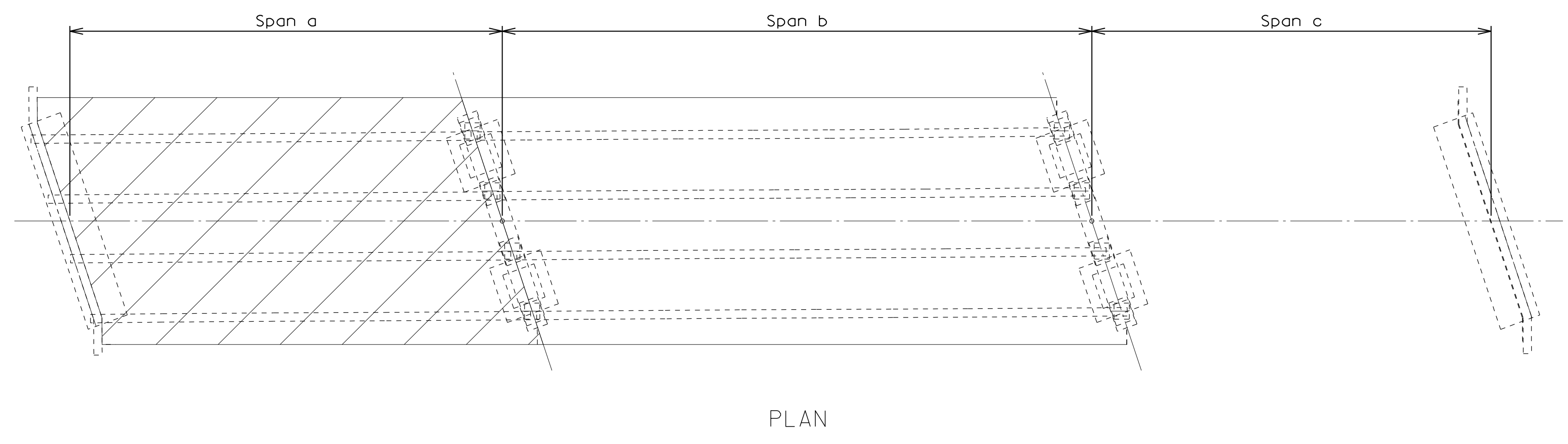
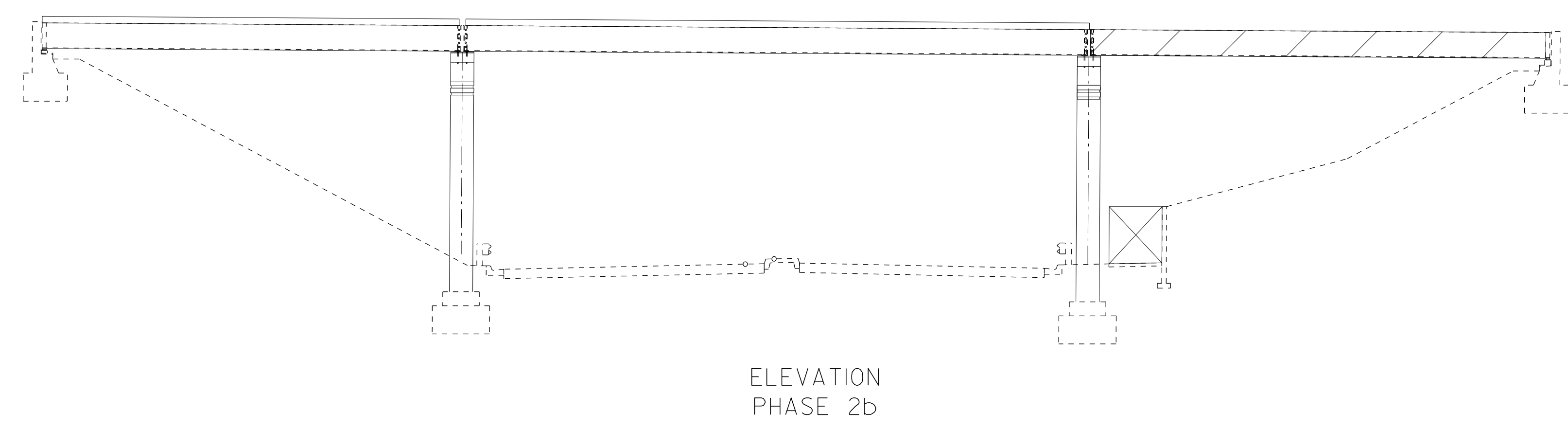


- PHASE 2b:**
1. Remove diaphragms in center bay.
  2. Remove Girders 1 and 2.
  3. Remove Girders 3 and 4.

- PHASE 3a:**
1. Remove slab on Span a.

**Note:** Contractor to check stability of Pier 2 prior to the removal of girders in Span c. Contractor to check stability of Pier 1 prior to the removal of slab on Span a.

**Note:**  
This is a schematic only. It is not a working drawing. The contractor shall submit complete details of the proposed erection methods and equipment in accordance with the specifications.



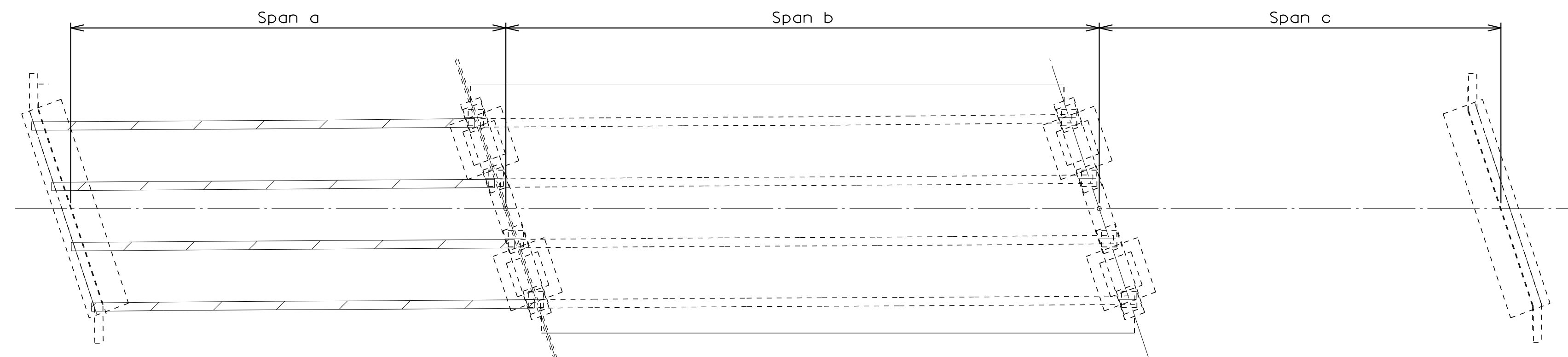
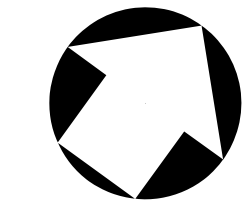
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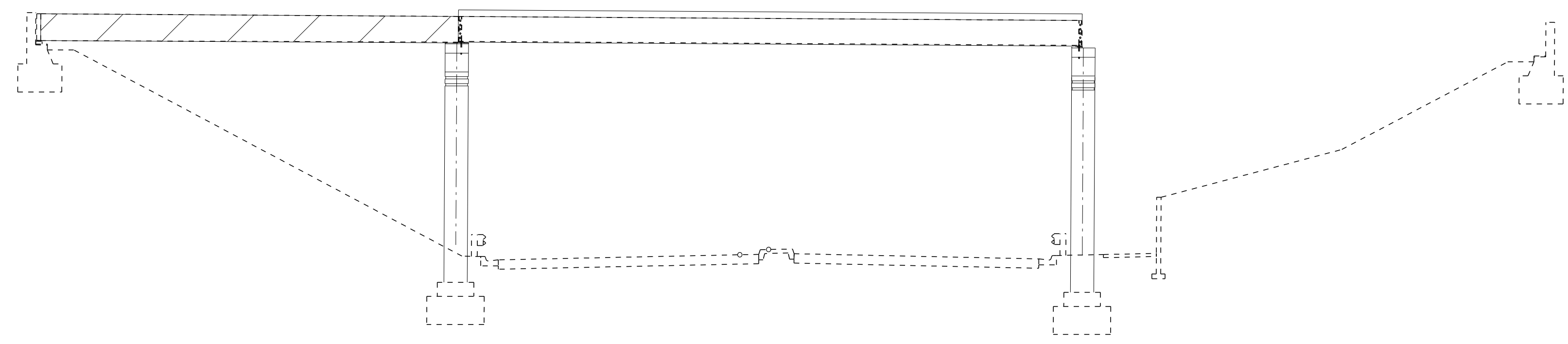
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CITY OF CHARLOTTESVILLE					
SEQUENCE OF CONSTRUCTION DEMOLITION PLAN 2 OF 4					
No.	Description	Date	Designed: KMR	Date	Plan No.
			Drawn: DAM	January	
			Checked: KMR	2026	
Revisions					Sheet No.
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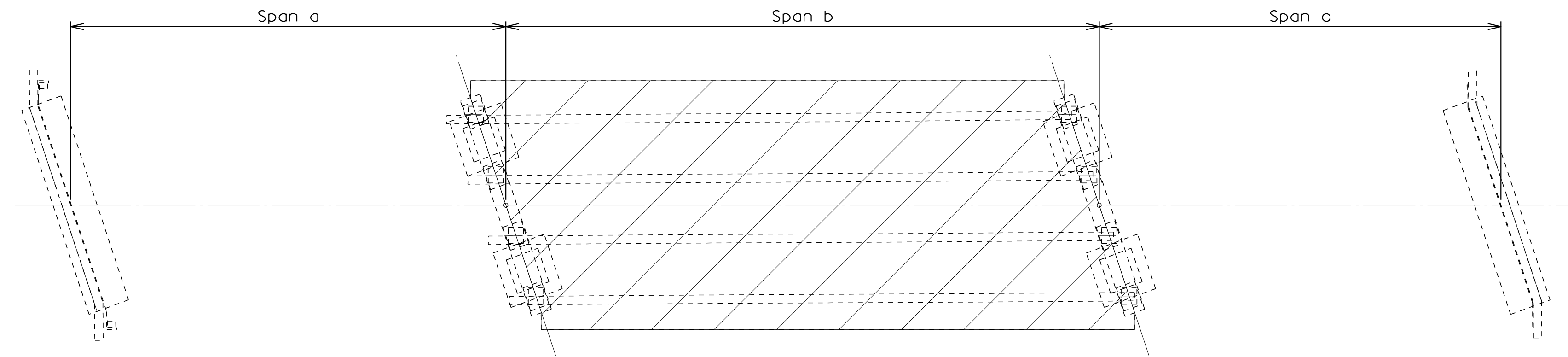
STATE	FEDERAL AID		STATE		SHEET
ROUTE	PROJECT		ROUTE	PROJECT	NO.
VA.	STP-5104 (326)		—	U000-104-365, B620, C501, P101	—



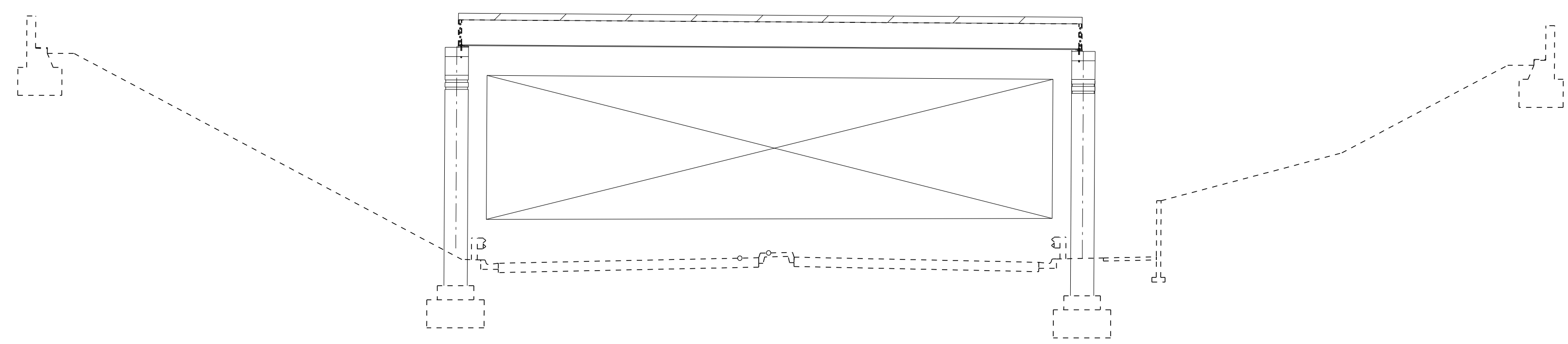
PLAN



ELEVATION  
PHASE 3b



PLAN



ELEVATION  
PHASE 4a

- PHASE 3b:**
1. Remove diaphragms in center bay.
  2. Remove Girders 1 and 2.
  3. Remove Girders 3 and 4.

- PHASE 4a:**
1. Close Route 250.
  2. Remove slab for Span b.

Note: Constructor to check stability of Pier 1 prior to the removal of girders in Span a.

Note:  
This is a schematic only. It is not a working drawing. The contractor shall submit complete details of the proposed erection methods and equipment in accordance with the specifications.

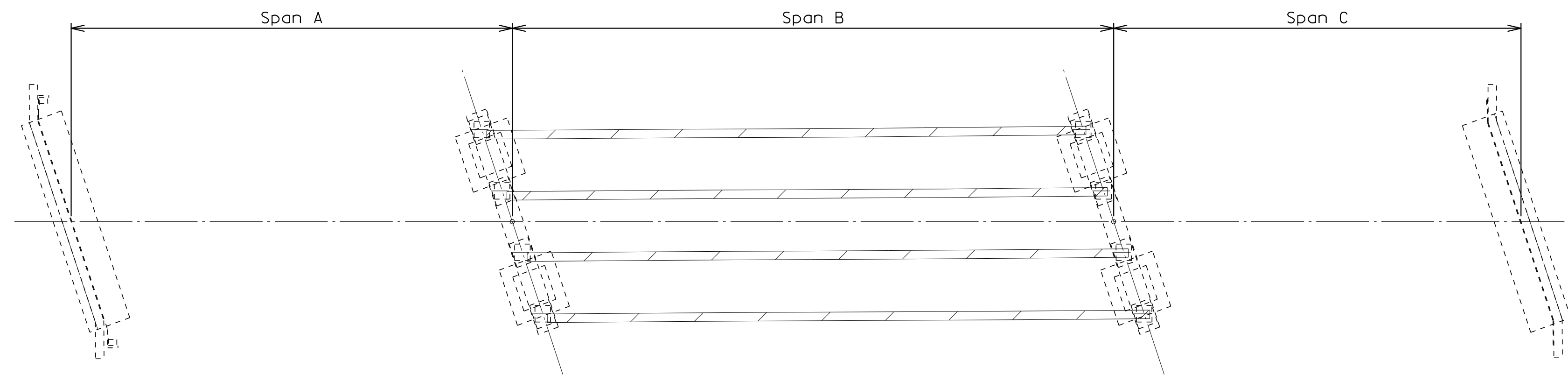
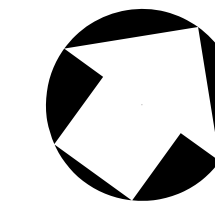
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SEQUENCE OF CONSTRUCTION DEMOLITION PLAN 3 OF 4					
No.	Description	Date	Designed: KMR	Date	Plan No.
			Drawn: DAM	January	
			Checked: KMR	2026	
Revisions					Sheet No.
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STATE	FEDERAL AID		STATE		SHEET
ROUTE	PROJECT		ROUTE	PROJECT	NO.
VA.	STP-5104 (326)		—	U000-104-365, B620, C501, P101	—



PLAN

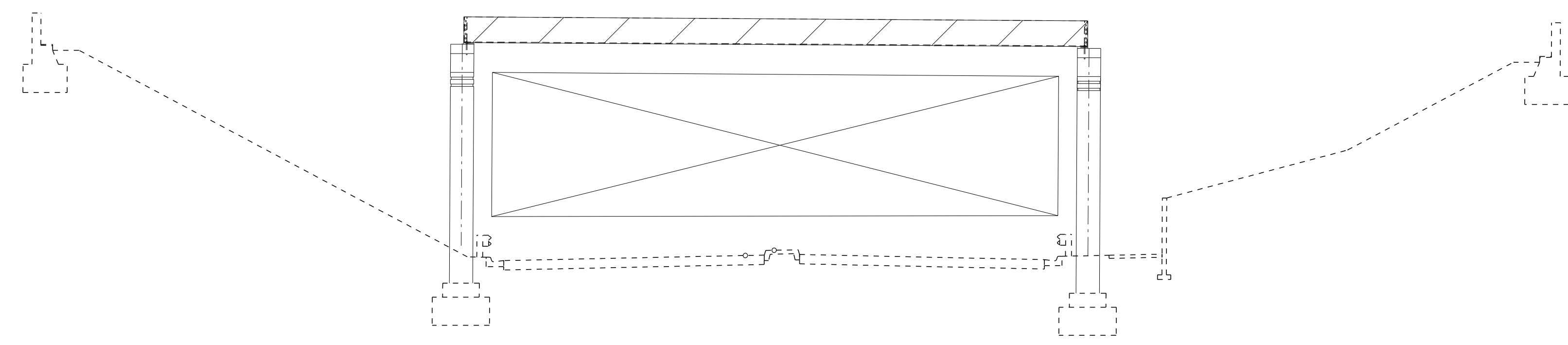
**PHASE 4b:**

1. Remove diaphragms in center bay.
2. Remove Girders 1 and 2.
3. Remove Girders 3 and 4.

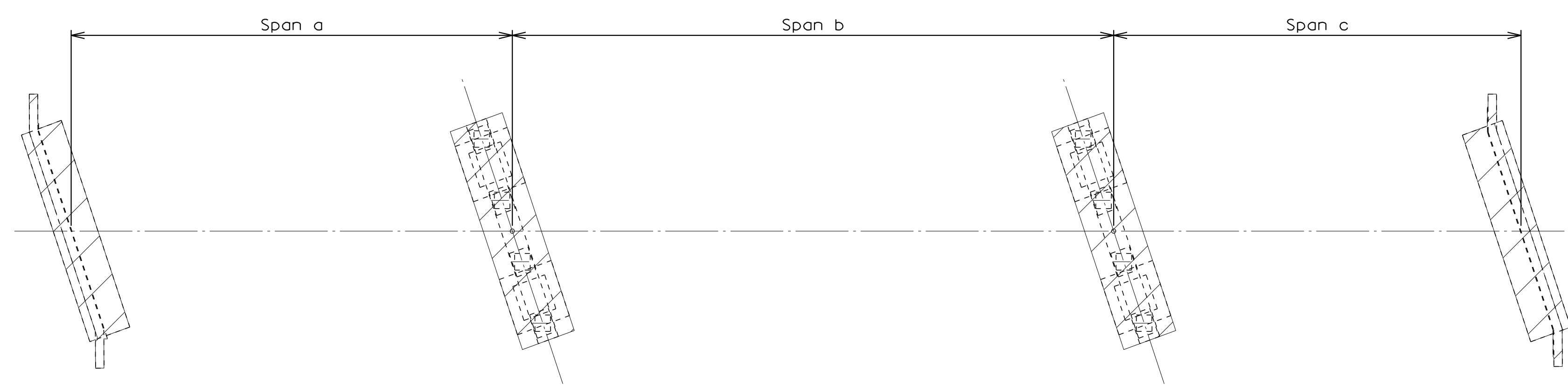
**PHASE 5:**

1. Demo Piers to 2ft below ground.
2. Remove Abutments.

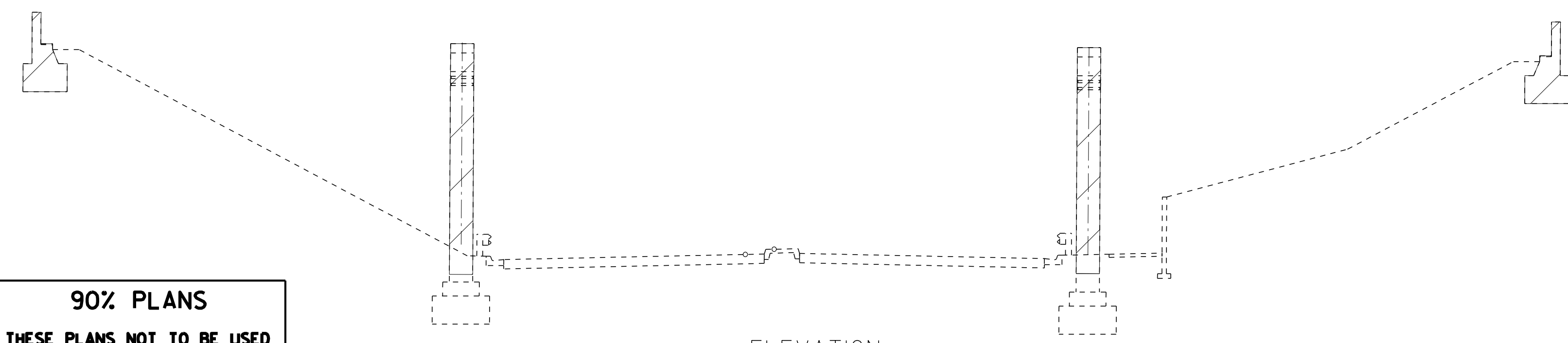
Note:  
This is a schematic only. It is not a working drawing. The contractor shall submit complete details of the proposed erection methods and equipment in accordance with the specifications.



ELEVATION  
PHASE 4b



PLAN



ELEVATION  
PHASE 5

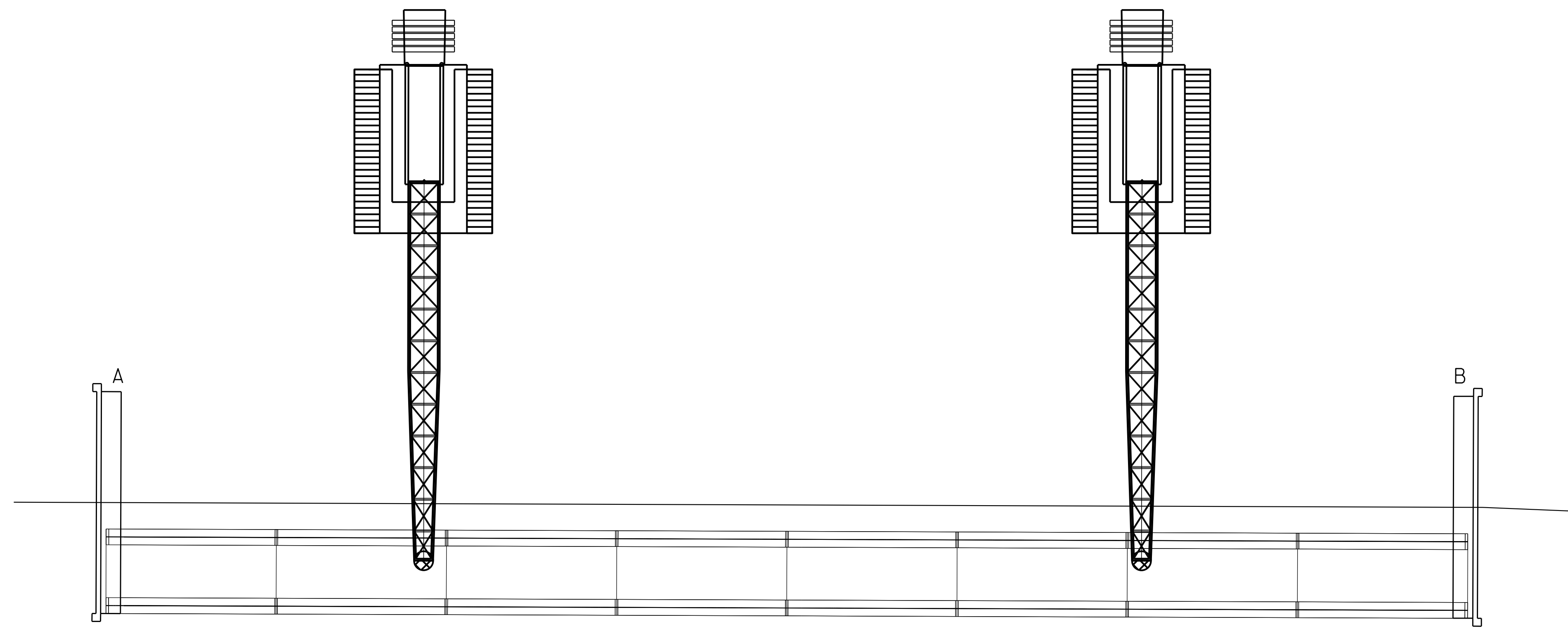
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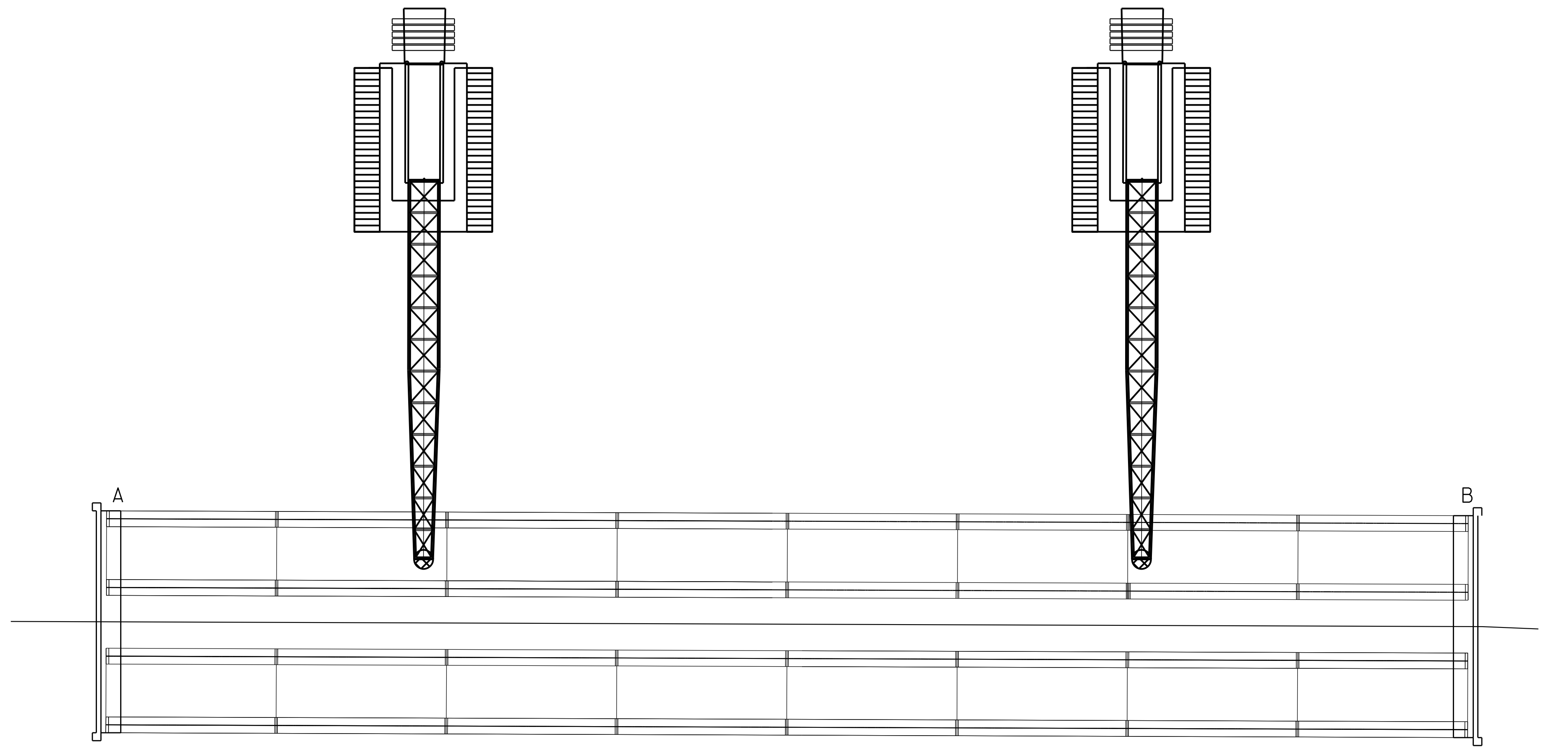
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CITY OF CHARLOTTESVILLE					
SEQUENCE OF CONSTRUCTION DEMOLITION PLAN 4 OF 4					
No.	Description	Date	Designed: KMR	Date	Plan No.
			Drawn: DAM	January	
			Checked: KMR	2026	
Revisions					Sheet No.
					8 of 30

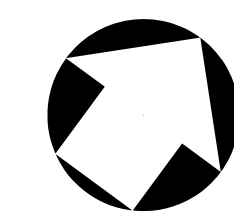
STATE	FEDERAL AID		STATE		SHEET
ROUTE	PROJECT		PROJECT		NO.
VA.	—	STP-5104 (326)	—	U000-104-365, B620, C501, P101	—



PLAN  
PHASE 2



PLAN  
PHASE 4



GENERAL NOTES:

Phase 1

1. Close Rte. 250 Bypass EB.
2. Deliver Girders 3 and 4.
3. Complete assembly of Girders 3 and 4 with Field Splices.
4. Install Cross Frames between Girders 3 and 4.

Phase 2

1. Close Rte. 250 Bypass EB and WB and Shared Use Path during nighttime closure allowance times.
2. Pick Girders 3 and 4 with Cross Frames using 2 - XXXXW Cranes - XXXft Boom. Min. : Boom Angle = 62 Deg., Oper. Rad. = XX'.
3. Secure Girders in place.
4. Open Rte. 250 Bypass EB and WB.

Phase 3

1. Close Rte. 250 Bypass EB.
2. Deliver Girders 1 and 2.
3. Complete assembly of Girders 1 and 2 with Field Splices.
4. Install Cross Frames between Girders 1 and 2.

Phase 4

1. Close Rte. 250 Bypass EB and WB and Shared Use Path during nighttime closure allowance times.
2. Pick Girders 1 and 2 with Cross Frames using 2 - XXXXW Cranes - XXXft Boom. Min. : Boom Angle = 62 Deg., Oper. Rad. = XX'.
2. Secure Girders in place.
4. Open Rte. 250 Bypass EB and WB.

Phase 5

1. Install Cross Frames between Girders 2 and 3. No installation is allowed over live traffic (pedestrian traffic included).
2. Install Stay-in-place forms between Girders 1, 2, 3, and 4. No installation is allowed over live traffic (pedestrian traffic included).

Erection will occur at night with a total lane closer of Route 250 Bypass.

Note:  
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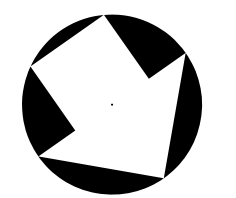
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SEQUENCE OF CONSTRUCTION ERECTION PLAN					
No.	Description	Date	Designed: KMR	Date	Plan No.
			Drawn: DAM	January	
			Checked: KMR	2026	
Revisions					Sheet No.
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STATE	FEDERAL AID		STATE		SHEET
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		STP-5104 (326)		U000-104-365, B620, C501, P101	---



Notes:

Portion of the integral abutment above the hinge shall be placed and cured to a minimum compressive strength of 3000 psi prior to the placement of the deck concrete.

Girders shall not be placed until girder support concrete gains a minimum strength of 3000 psi.

Backfill behind integral abutment footing shall not be placed until concrete in upper portion of the integral abutment attains a minimum compressive strength of 3000 psi. The remainder of the backfill behind both bridge abutments shall be placed such that the differential in the height of fill at each abutment shall not exceed 6".

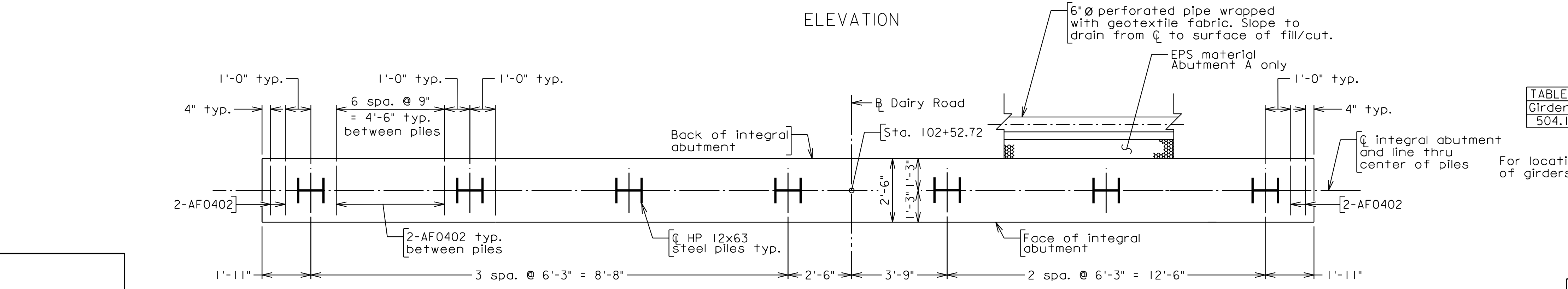
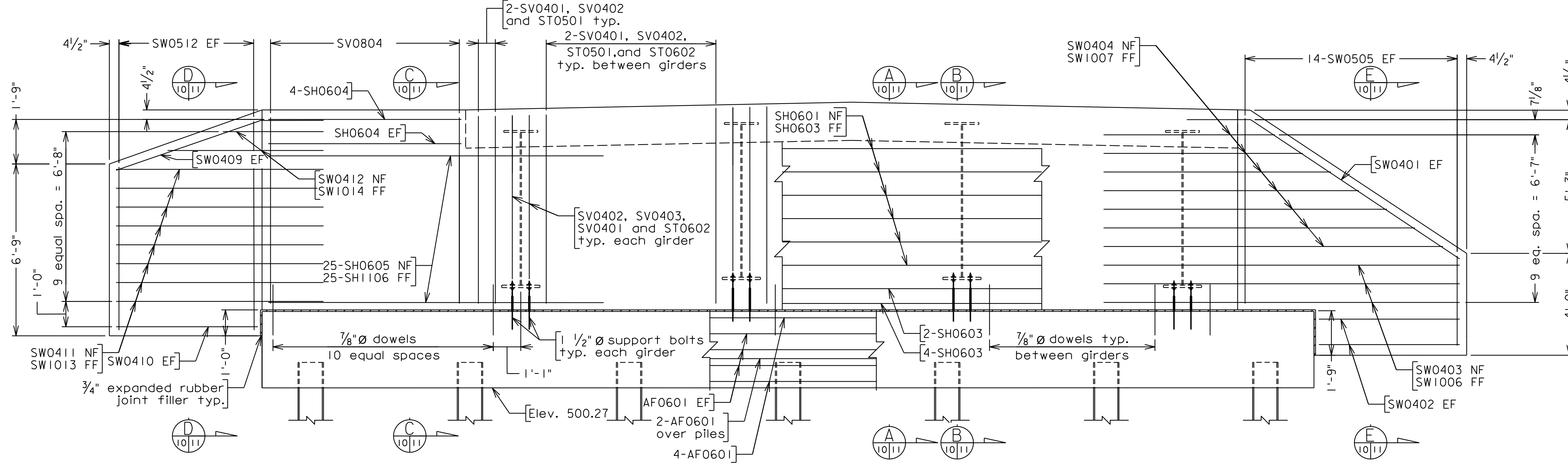
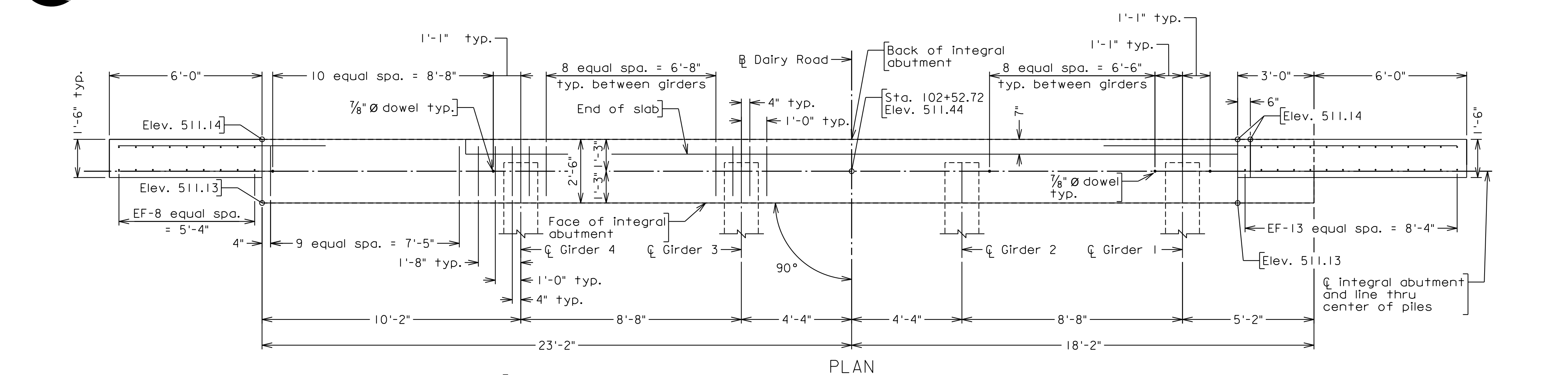
The portion of abutment above the hinge shall be cast when the least thermal movement of the superstructure can be expected during the period of initial set of concrete, e.g. at dusk or during an expected uniformly cloudy day.

Tolerance for driving piles shall be specified for "Column supports for bent caps" in Section 403, Table IV-1 of the Specifications.

See sheet 15 for end of girder details.

All chamfers shall be 3/4".

Elevations in plan view are at top of slab.



Girder	1	2	3	4
Elevation	504.12	504.29	504.29	504.12

For location of Point of Elevation on bottom of girders, see Support Bolt Detail, sheet 14.

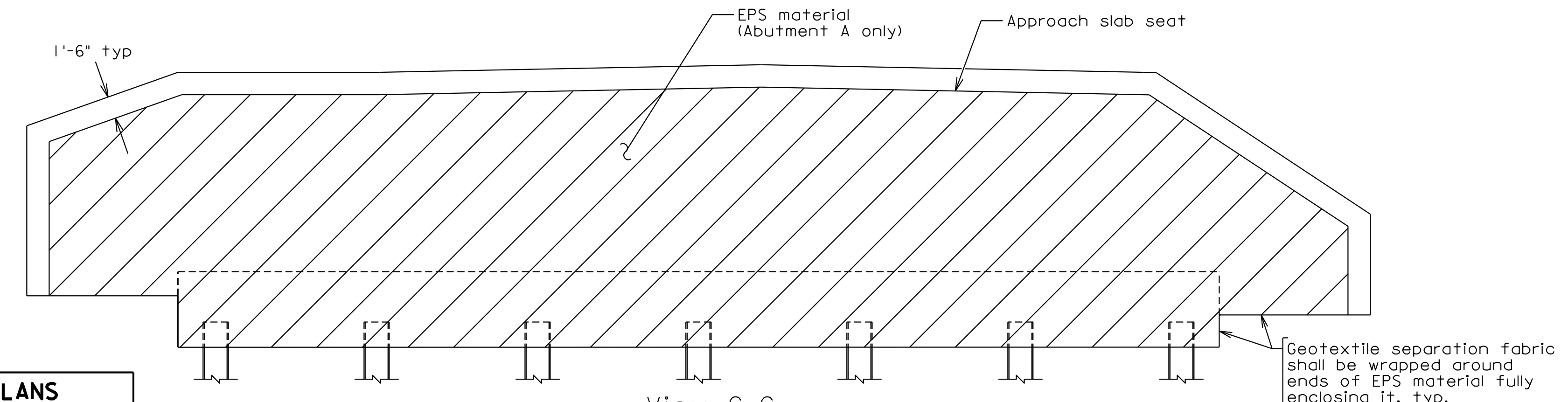
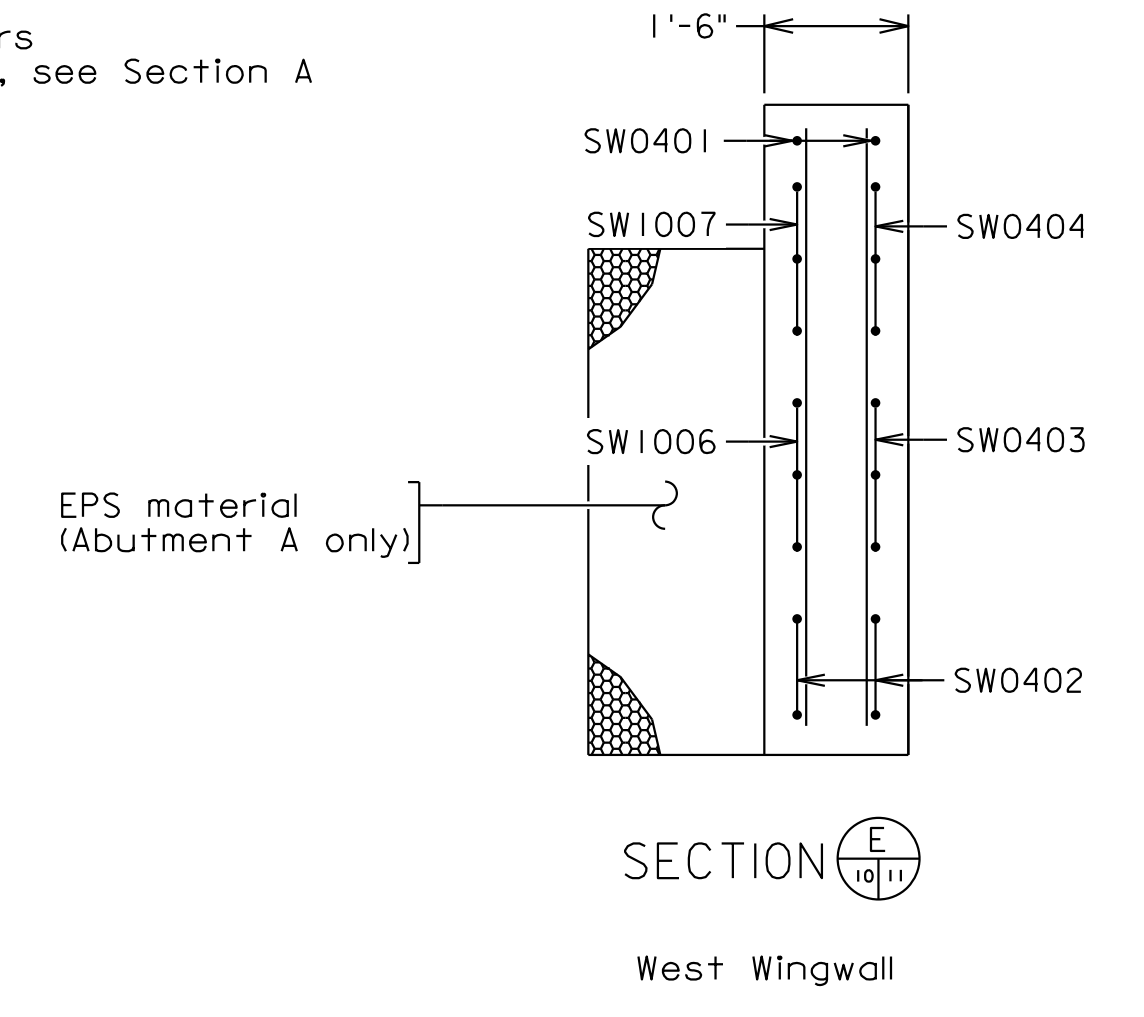
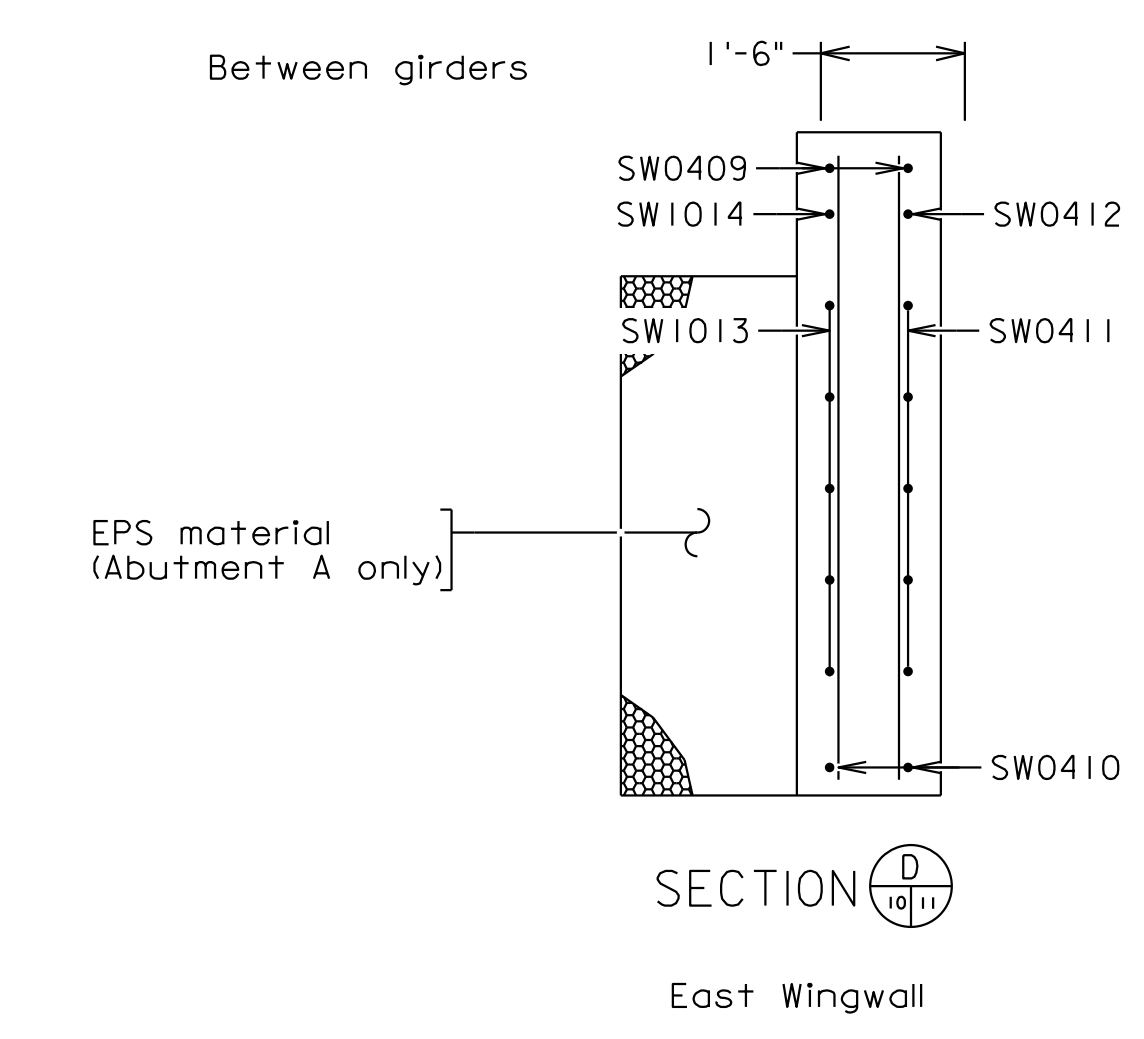
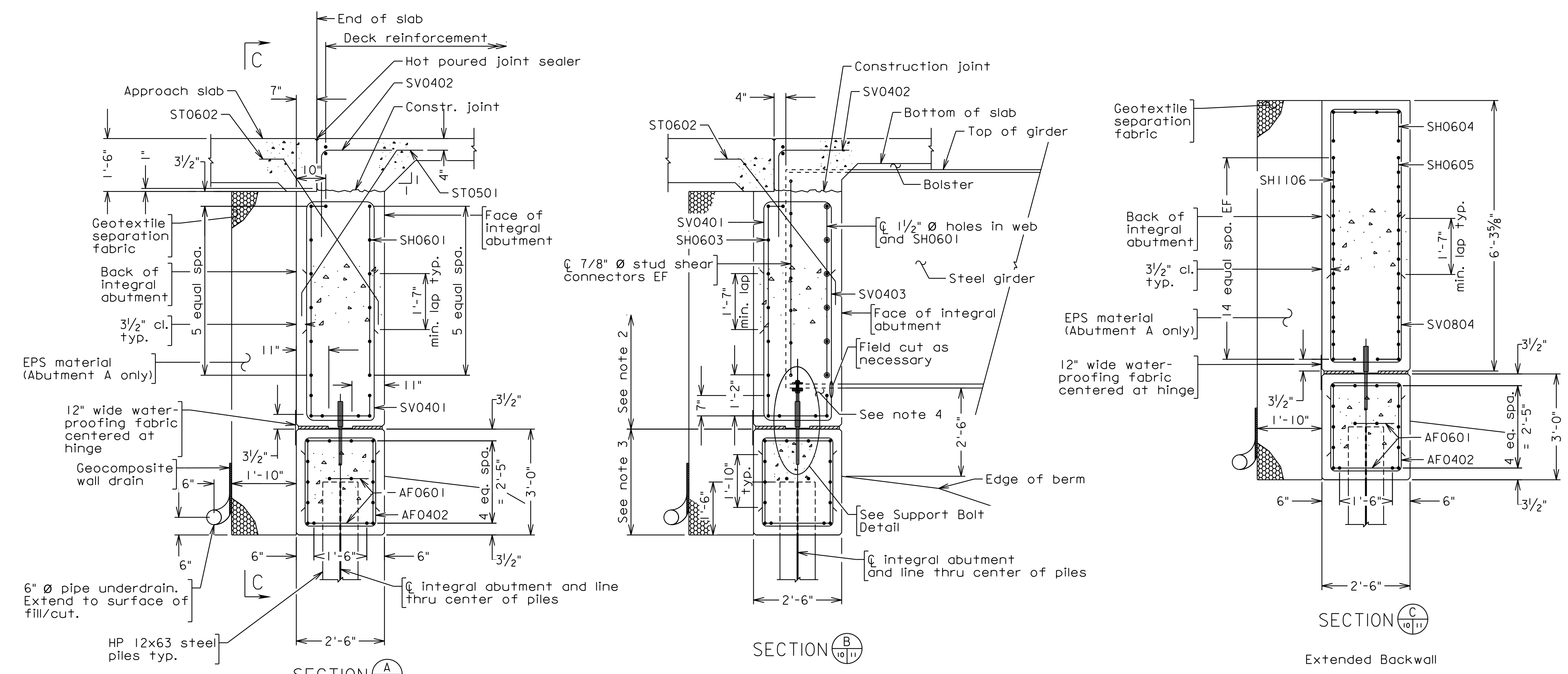
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PILE AND FOOTING REINFORCEMENT PLAN  
 AF0601 bars are not shown for clarity

CITY OF CHARLOTTESVILLE				
ABUTMENT A				
No.	Description	Date	Designed: GSC Drawn: DAM Checked: LCH	Date: January 2026
Revisions			Plan No.	Sheet No.
				10 OF 30

STATE	FEDERAL AID	STATE	SHEET NO.
VA.	PROJECT STP-5104 (326)	PROJECT U000-104-365, B620, C501, P101	11 of 30

- Notes:
- All Chamfers shall be 3/4".
  - Low Shrinkage Class A4 Modified concrete and Corrosion Resistant Reinforcing (CRR) steel, same Class as deck, is included with superstructure quantities.
  - Class A3 concrete and CRR steel, Class 1, is included with sub-structure quantities.
  - Extreme care must be taken, when placing concrete, to eliminate any voids under girder flanges.
  - Costs of dowels shall be included in the bid price for structural steel. Cost of neoprene and roofing felt are to be included in other bid items.



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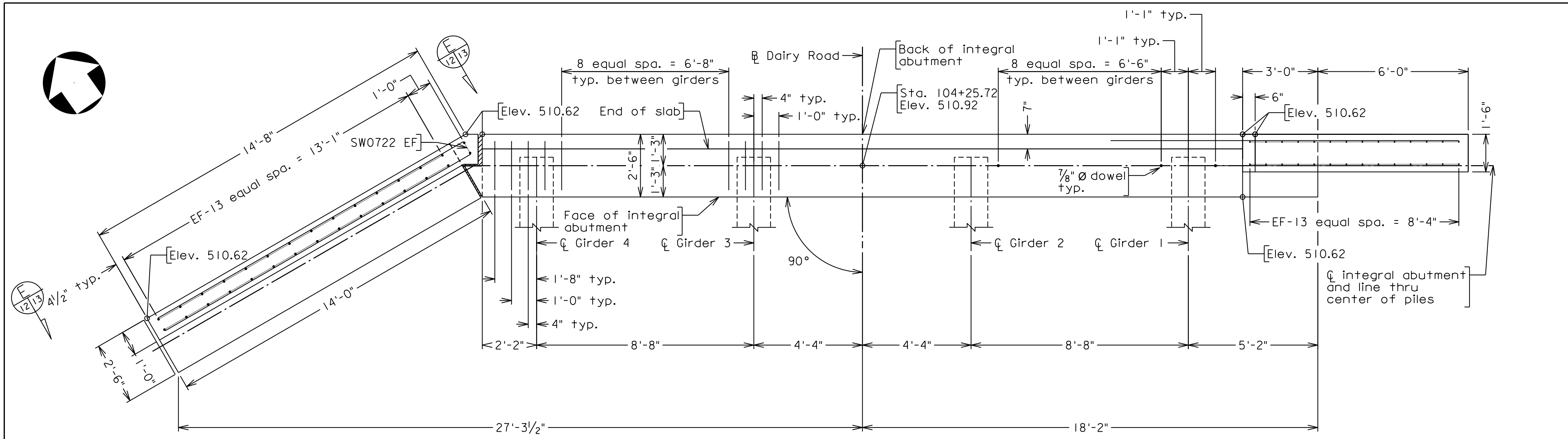
View C-C  
 Abut A shown  
 Not to scale

Scale: 1/2" = 1'-0" © 2026, Commonwealth of Virginia

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		CITY OF CHARLOTTESVILLE	
		ABUTMENT A SECTIONS AND DETAILS	
No.	Description	Date	Designed: GSC Drawn: DAM Checked: LCH
	Revisions	January 2026	Plan No. Sheet No. 11 of 30

STATE	FEDERAL AID	STATE	SHEET NO.
VA.	PROJECT STP-5104 (326)	ROUTE U000-104-365, B620, C501, P101	



PLAN

Notes:

Portion of the integral abutment above the hinge shall be placed and cured to a minimum compressive strength of 3000 psi prior to the placement of the deck concrete.

Girders shall not be placed until girder support concrete gains a minimum strength of 3000 psi.

Backfill behind integral abutment footing shall not be placed until concrete in upper portion of the integral abutment attains a minimum compressive strength of 3000 psi. The remainder of the backfill behind both bridge abutments shall be placed such that the differential in the height of fill at each abutment shall not exceed 6".

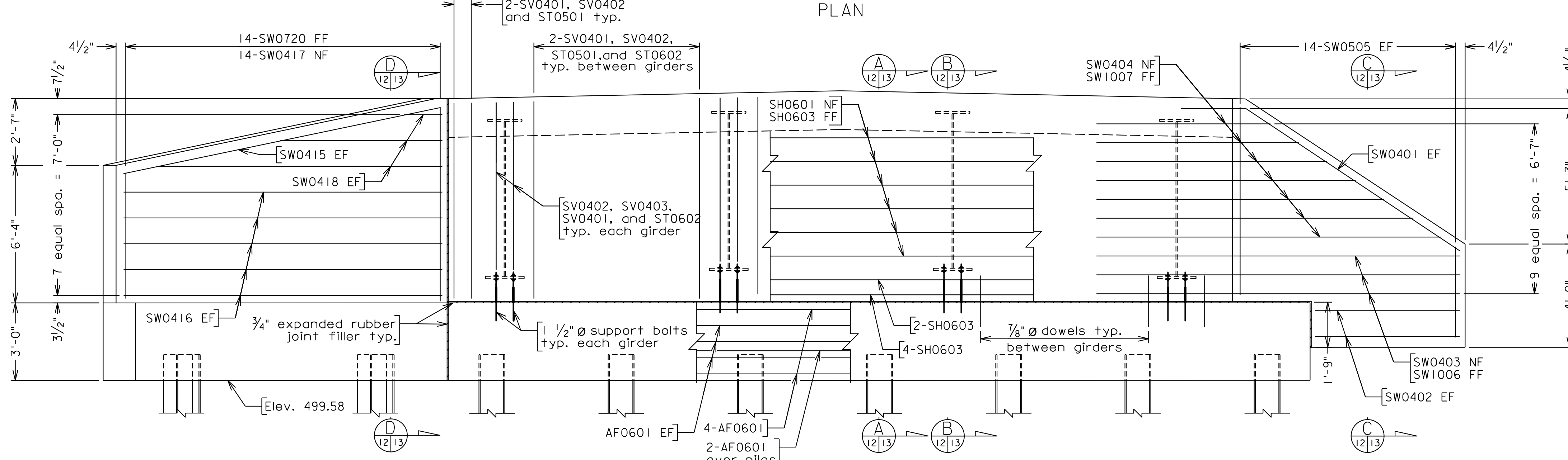
The portion of abutment above the hinge shall be cast when the least thermal movement of the superstructure can be expected during the period of initial set of concrete, e.g. at dusk or during an expected uniformly cloudy day.

Tolerance for driving piles shall be specified for "Column supports for bent caps" in Section 403, Table IV-1 of the Specifications.

See sheet 15 for end of girder details.

All chamfers shall be 3/4".

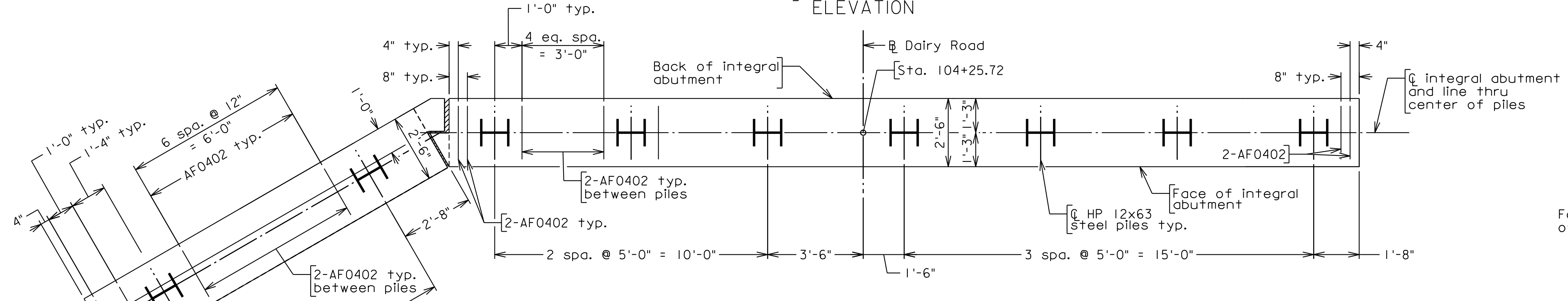
Elevations in plan view are at top of slab.



ELEVATION

Girder	1	2	3	4
Elevation	503.60	503.77	503.77	503.60

For location of Point of Elevation on bottom of girders, see Support Bolt Detail, sheet 14.



PILE AND FOOTING REINFORCEMENT PLAN  
AF0601 bars are not shown for clarity

**90% PLANS**  
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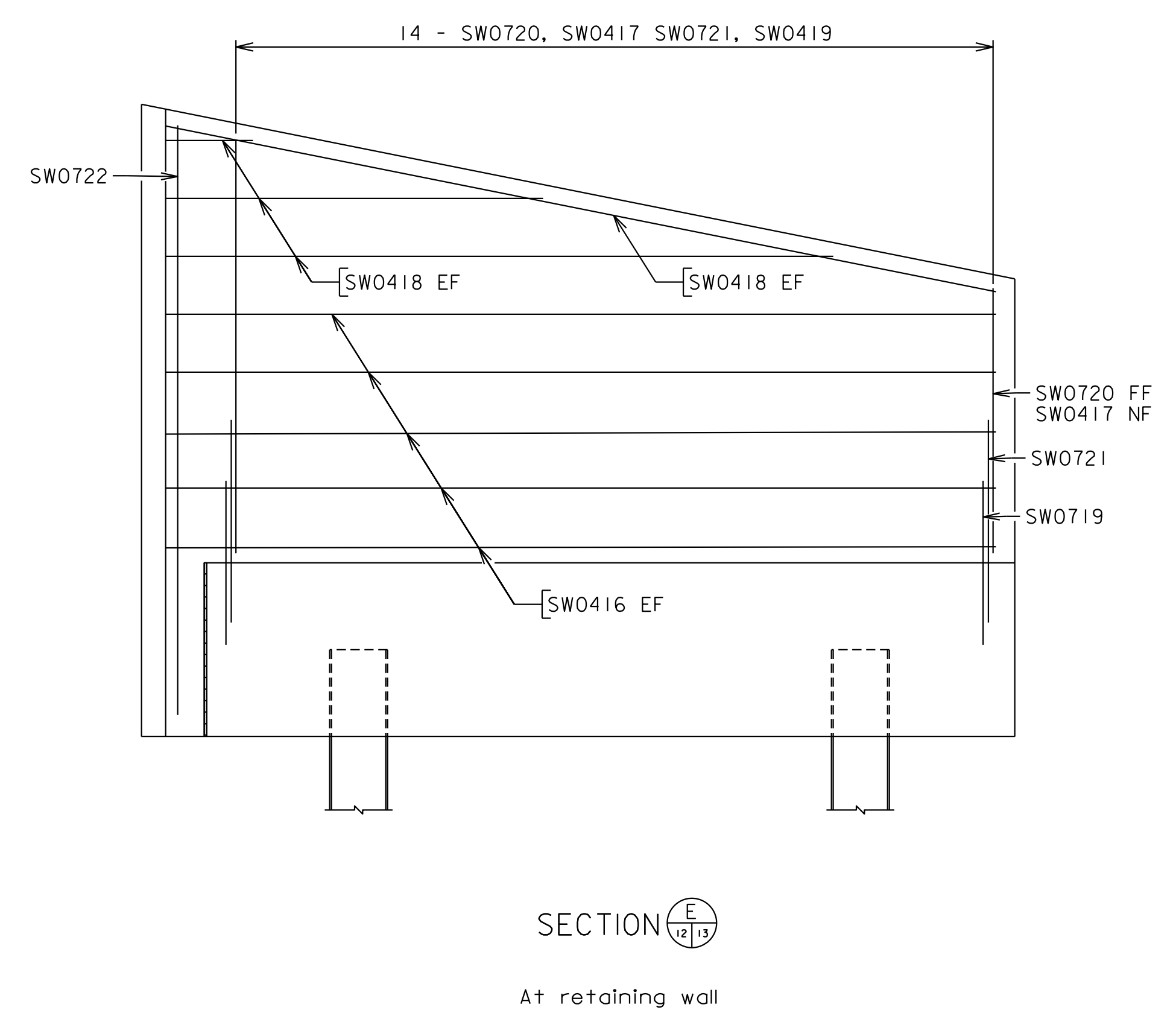
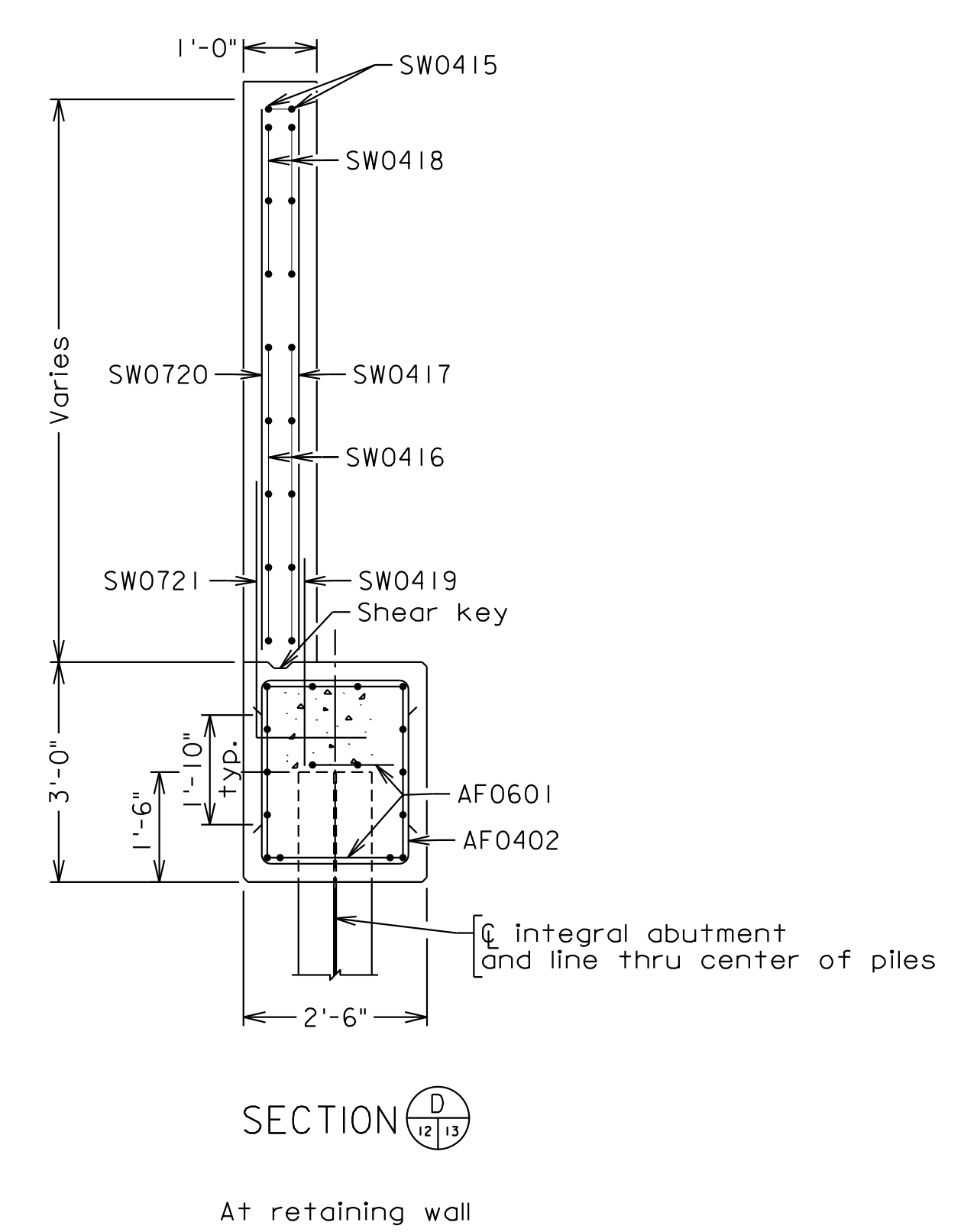
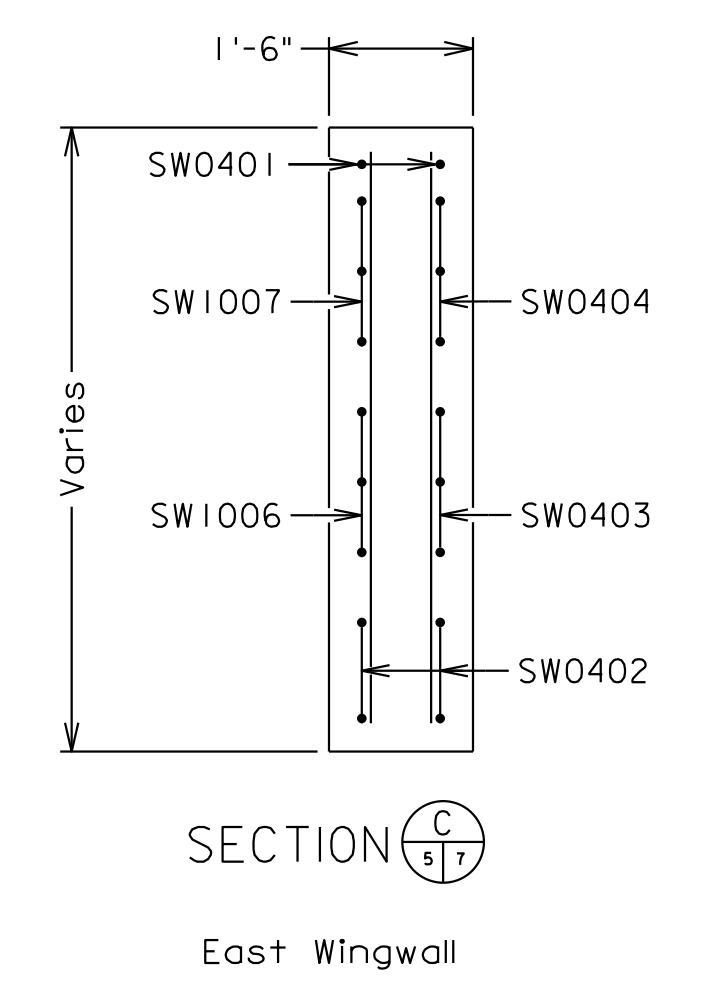
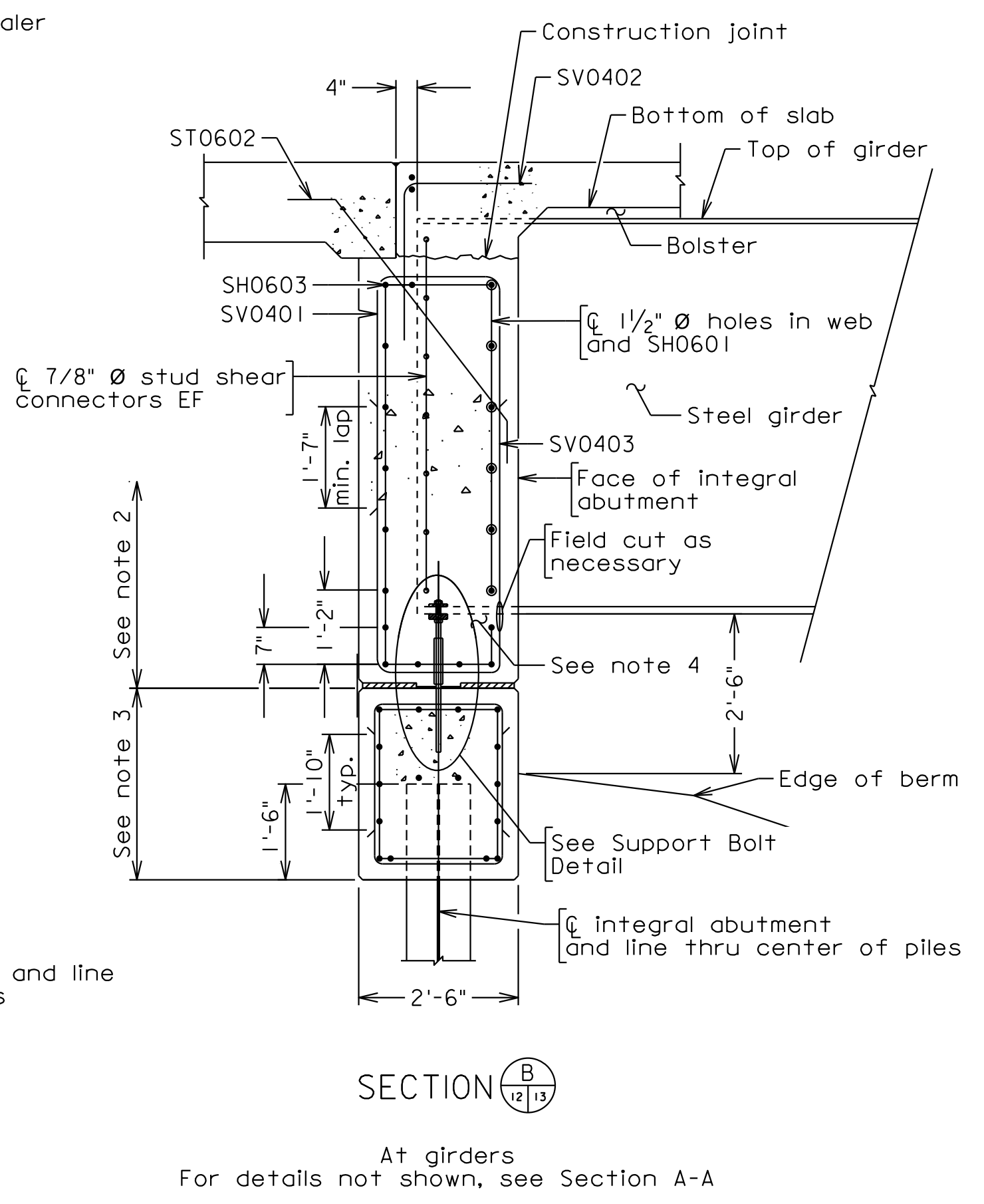
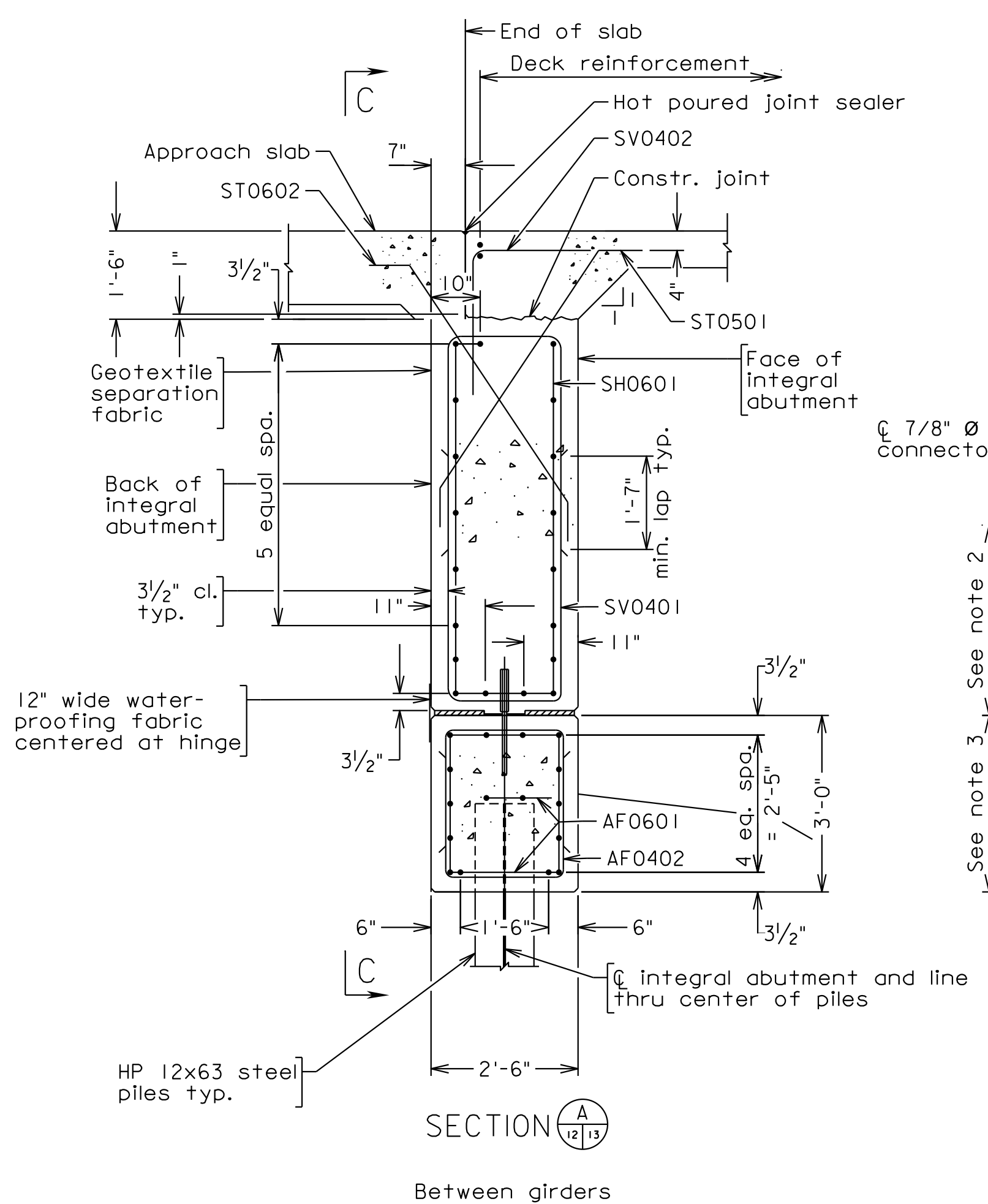
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CITY OF CHARLOTTESVILLE					
ABUTMENT B					
No.	Description	Date	Designed: GSC	Date	Plan No.
			Drawn: DAM	January	
			Checked: LCH	2026	
Revisions					Sheet No.
					12 of 30

STATE	FEDERAL AID	STATE	SHEET NO.
VA.	PROJECT STP-5104 (326)	ROUTE PROJECT U000-104-365, B620, C501, P101	13

- Notes:
- All Chamfers shall be 3/4".
  - Low Shrinkage Class A4 Modified concrete and Corrosion Resistant Reinforcing (CRR) steel, same Class as deck, is included with superstructure quantities.
  - Class A3 concrete and CRR steel, Class 1, is included with sub-structure quantities.
  - Extreme care must be taken, when placing concrete, to eliminate any voids under girder flanges.
  - Costs of dowels shall be included in the bid price for structural steel. Cost of neoprene and roofing felt are to be included in other bid items.



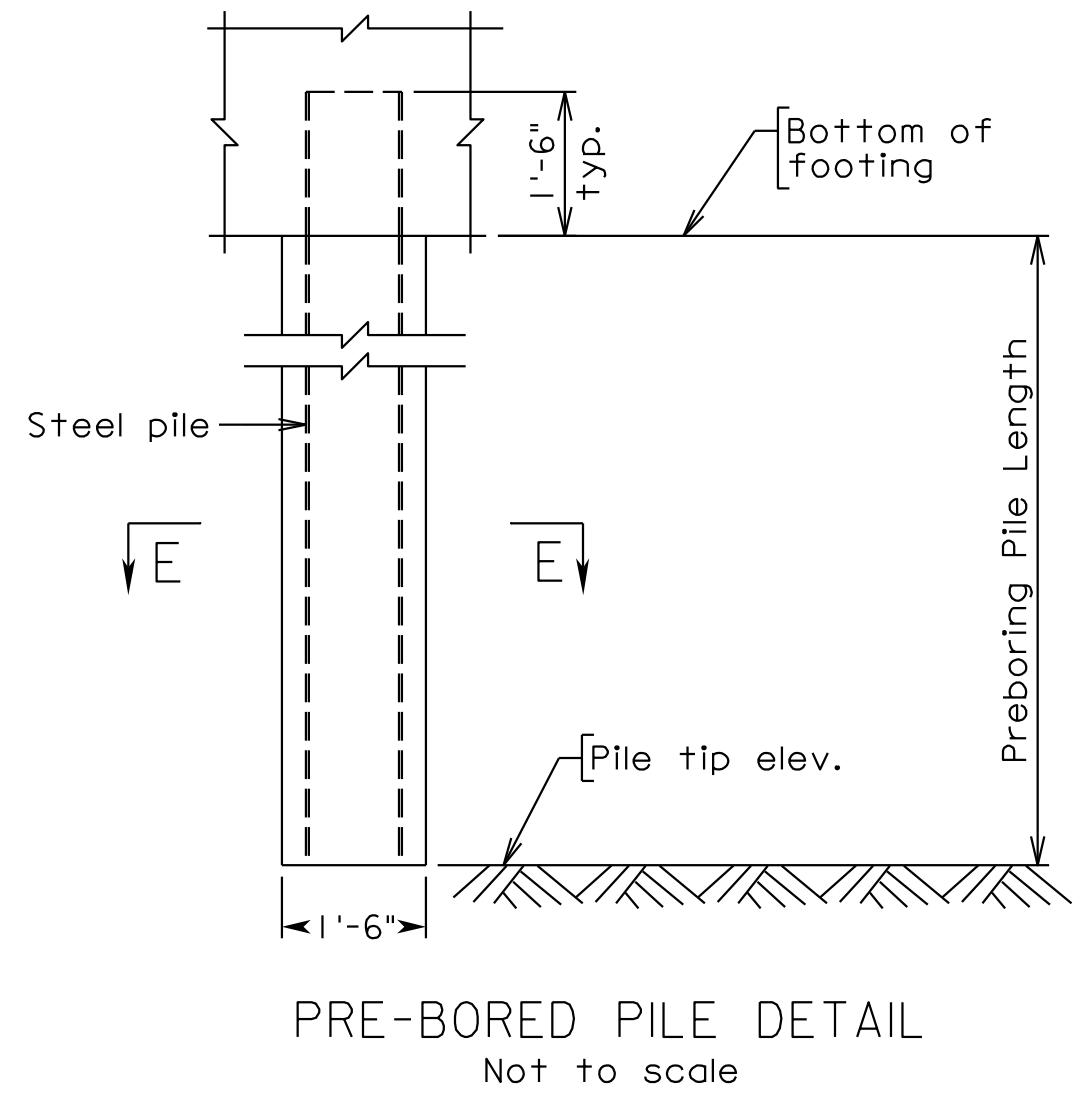
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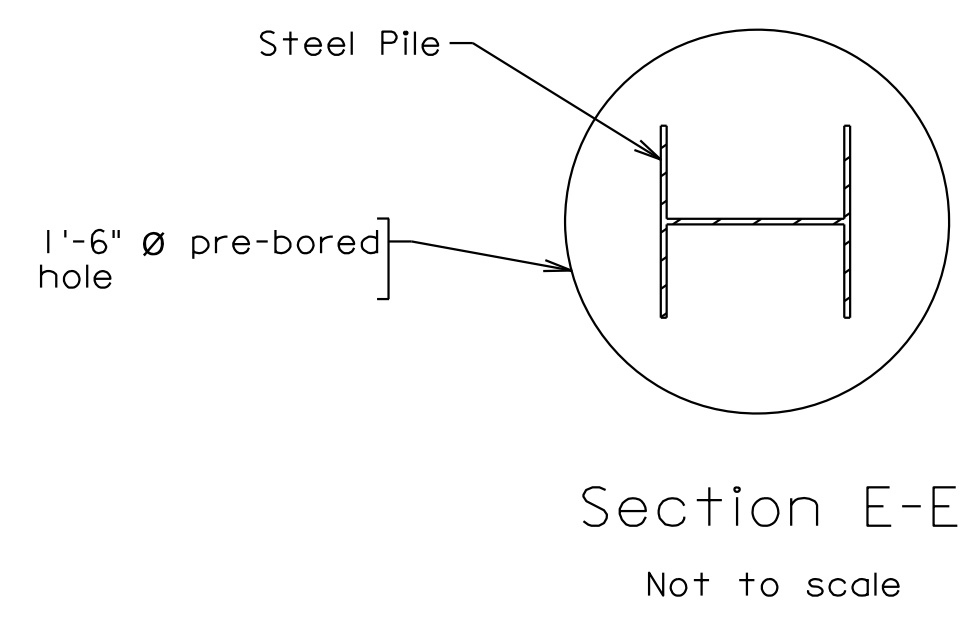
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		CITY OF CHARLOTTESVILLE	
		ABUTMENT B SECTIONS AND DETAILS	
No.	Description	Date	Sheet No.
	Revisions	Designed: GSC Drawn: DAM Checked: LCH	13 of 30
		Date: January 2026	Plan No.

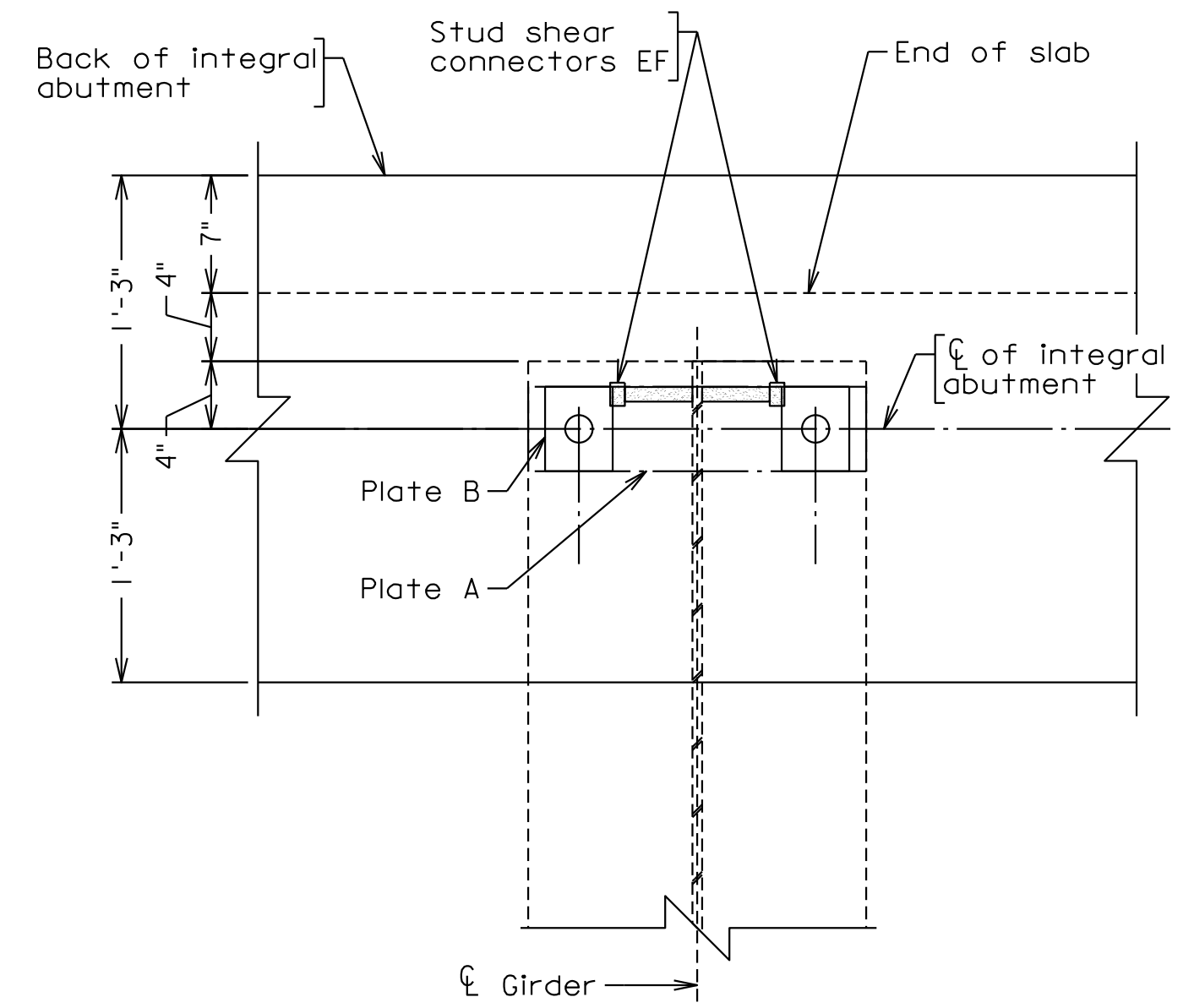
STATE	FEDERAL AID		STATE		SHEET NO.
VA.	ROUTE	PROJECT	ROUTE	PROJECT	
		STP-5104 (326)		U000-104-365, B620, C501, P101	



PRE-BORED PILE DETAIL  
Not to scale

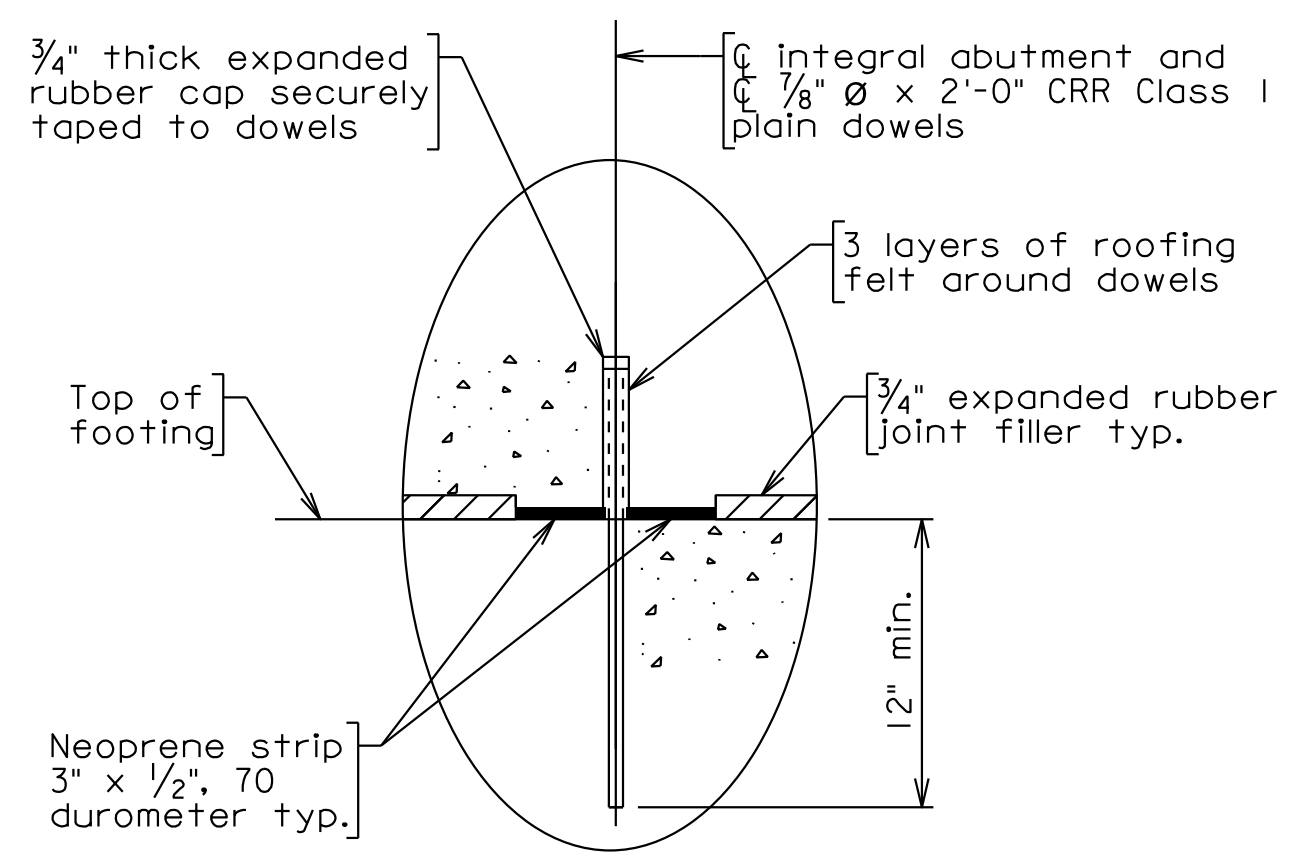


Section E-E  
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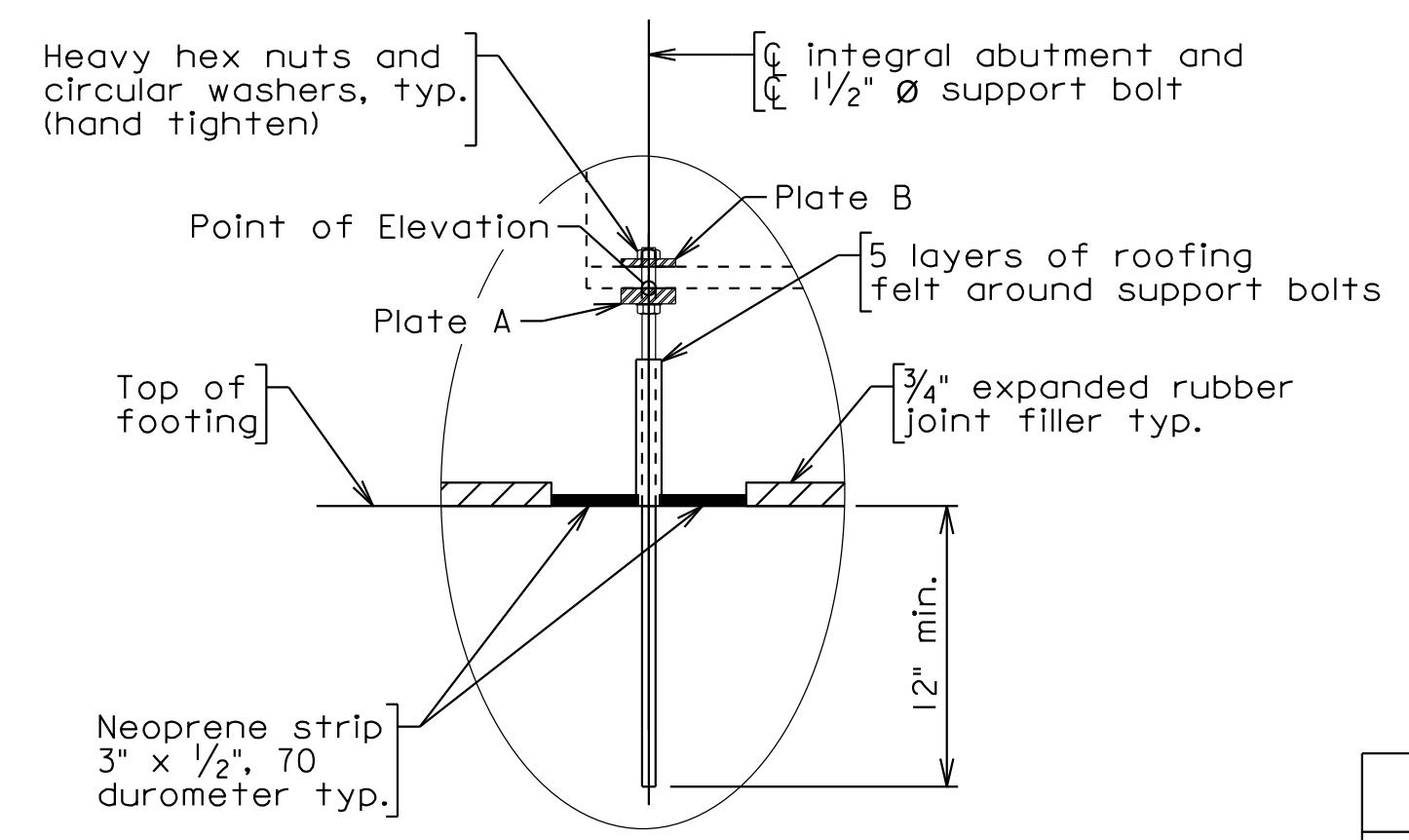


TYPICAL DETAIL FOR GIRDER  
END AT ABUTMENTS  
Not to scale

- Notes:
1. All Chamfers shall be 3/4".
  2. Low Shrinkage Class A4 Modified concrete and Corrosion Resistant Reinforcing (CRR) steel, same Class as deck, is included with superstructure quantities.
  3. Class A3 concrete and CRR steel, Class I, is included with substructure quantities.
  4. Extreme care must be taken, when placing concrete, to eliminate any voids under girder flanges.
  5. Costs of dowels shall be included in the bid price for structural steel. Cost of neoprene and roofing felt are to be included in other bid items.



HINGE DETAIL  
Not to scale



SUPPORT BOLT DETAIL  
Not to scale

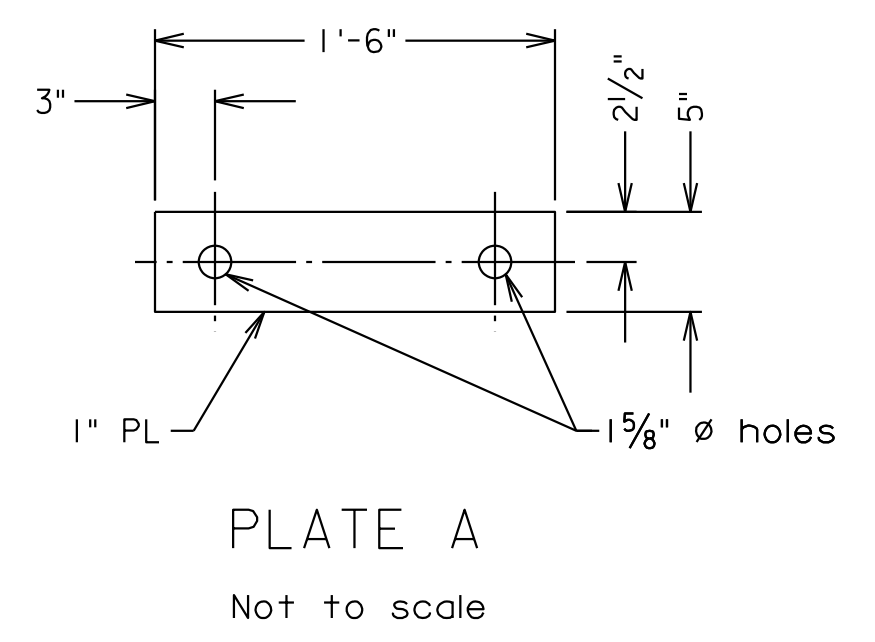


PLATE A  
Not to scale

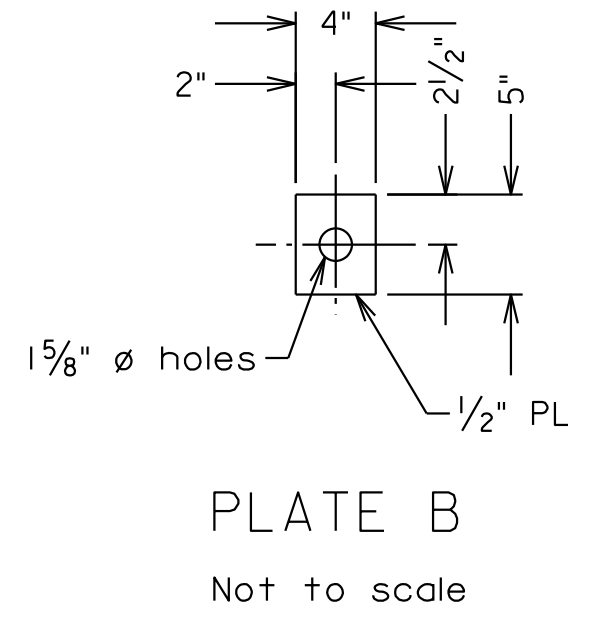


PLATE B  
Not to scale

Substructure Unit	Rock Socket Bearing Material	Rock Socket Length (ft)	Class T3 Concrete Fill Elevation (ft)	Factored Axial Resistance (Tons/pile)	Tip Elevation (ft)	Steel Pile Size	Pre-Bored Hole Diameter (ft)
Abutment A	N/A	N/A	N/A	184	475.27	12x63	1'-6"
Abutment B	N/A	N/A	N/A	184	*	12x63	1'-6"

The Strength Limit State controls the design.

- \* IGM and Rock elevations vary across the abutment. Pile tip elevations are anticipated between 474.58 (B-07) and 469.00 (B-08). Piles to be driven to refusal a minimum of 5 feet into IGM or Rock.

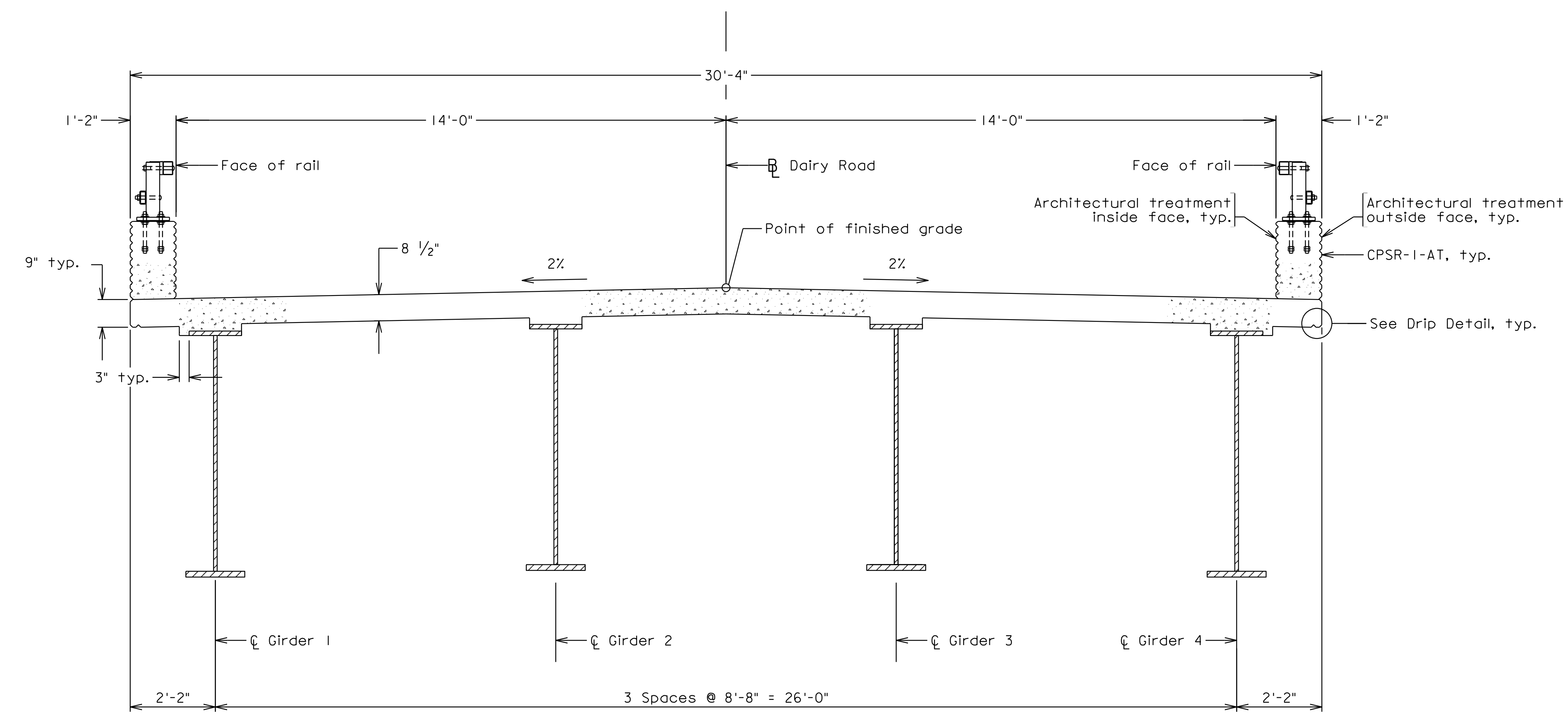
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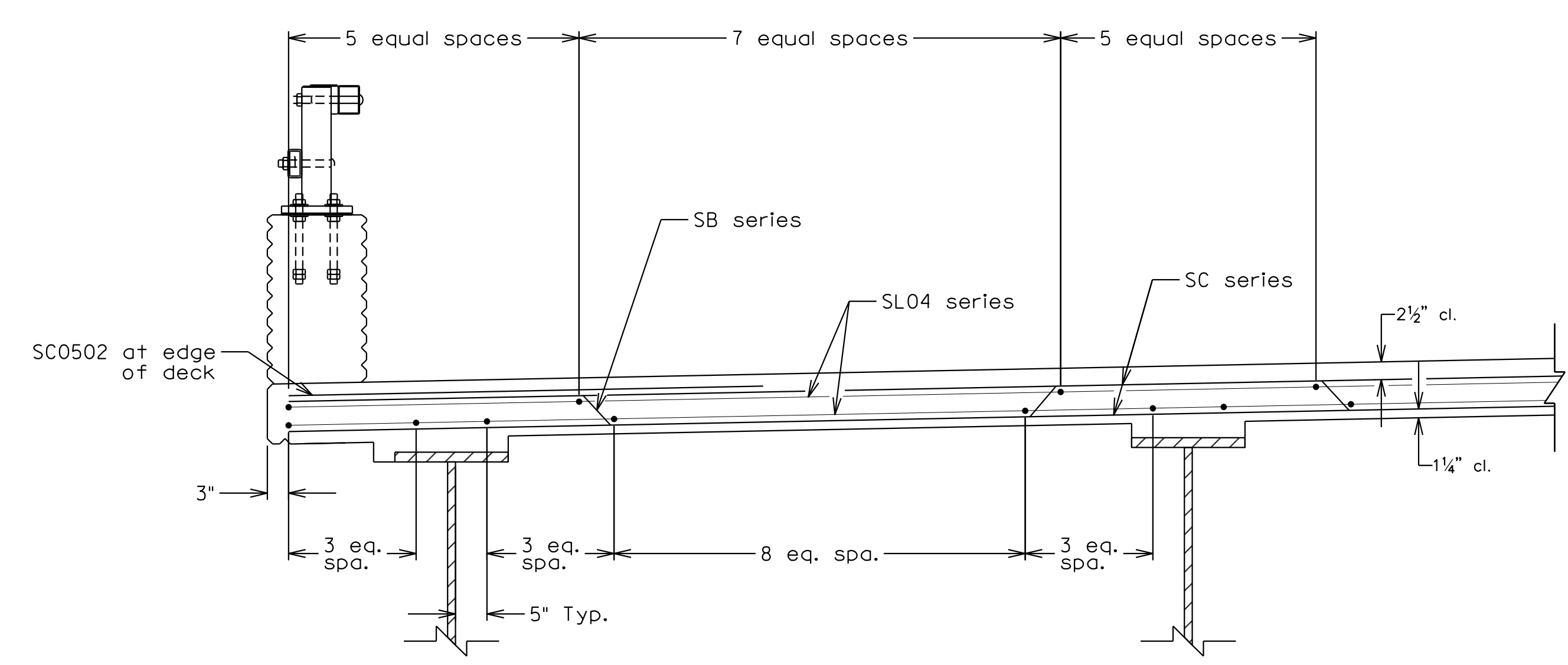
CITY OF CHARLOTTESVILLE			
MISCELLANEOUS ABUTMENT DETAILS			
No.	Description	Date	Designed: GSC Drawn: DAM Checked: LCH
Revisions		Date January 2026	Plan No. Sheet No. 14 of 30

STATE	FEDERAL AID	STATE	SHEET NO.
VA.	PROJECT	ROUTE	PROJECT
	STP-5104 (326)		U000-104-365, B620, C501, P101

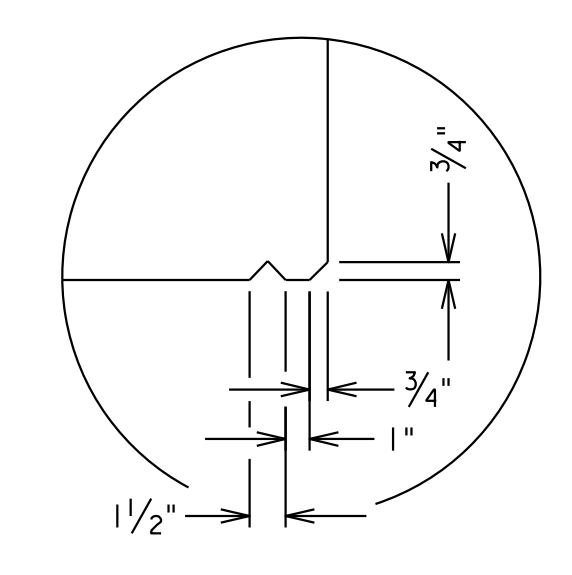


TRANSVERSE SECTION

Notes:  
 For additional reinforcing steel details, see Deck Plan on sheet 22.  
 For rail details, see sheet 23.



PART SECTION AT MIDSPAN  
 SCALE: 1/2" = 1'-0"



DETAIL  
 SCALE: 1/2" = 1'-0"

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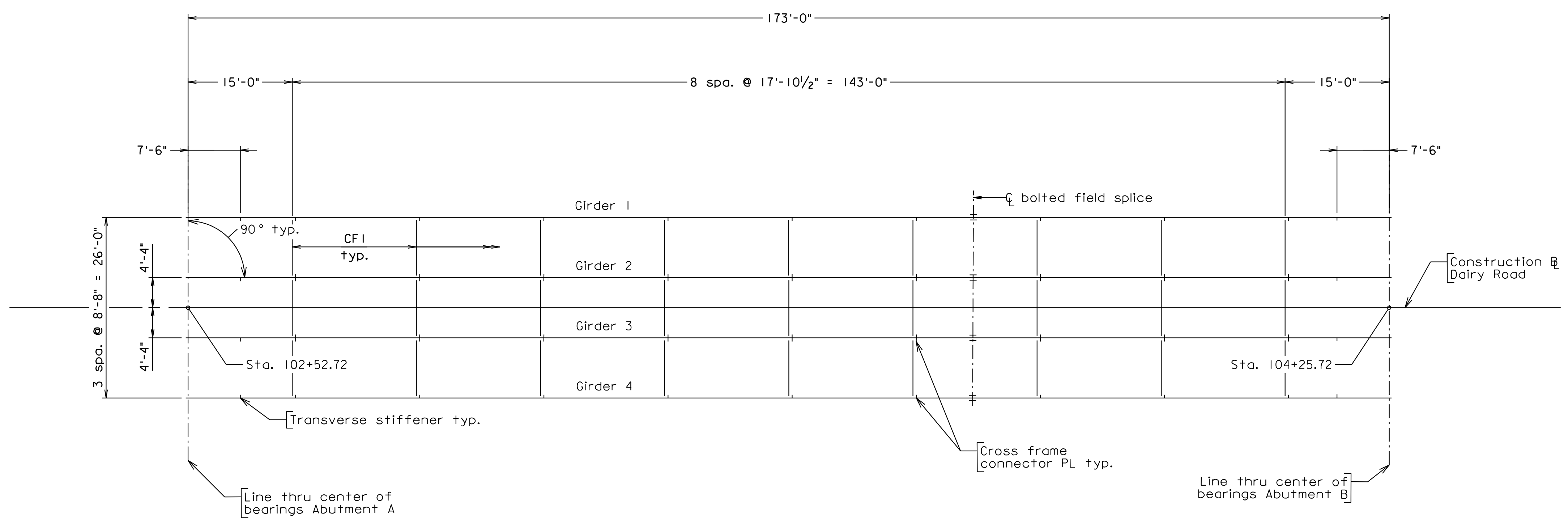
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		CITY OF CHARLOTTESVILLE	
		TRANSVERSE SECTION	
No.	Description	Date	Sheet No.
	Revisions		15 of 30
Designed: KMR	Drawn: DAM	Checked: GSC	Date: January 2026
Plan No.			

STATE	FEDERAL AID		STATE		SHEET
ROUTE	PROJECT		ROUTE	PROJECT	NO.
VA.	STP-5104 (326)		—	U000-104-365, B620, C501, P101	—

Notes:  
 For bolted splice details, see Sheet 18.  
 For girder details, see Sheet 17.



FRAMING PLAN

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CITY OF CHARLOTTESVILLE					
FRAMING PLAN					
No.	Description	Date	Designed: KMR	Date	Plan No.
			Drawn: DAM	January	
			Checked: DGD	2026	
Revisions					Sheet No.
					16 of 30

STATE	FEDERAL AID		STATE		SHEET NO.
	ROUTE	PROJECT	ROUTE	PROJECT	
VA.	---	STP-5104 (326)	---	U000-104-365, B620, C501, P101	---



GIRDER ELEVATION  
Scale: 1/8" = 1'-0" horizontal only

Notes:

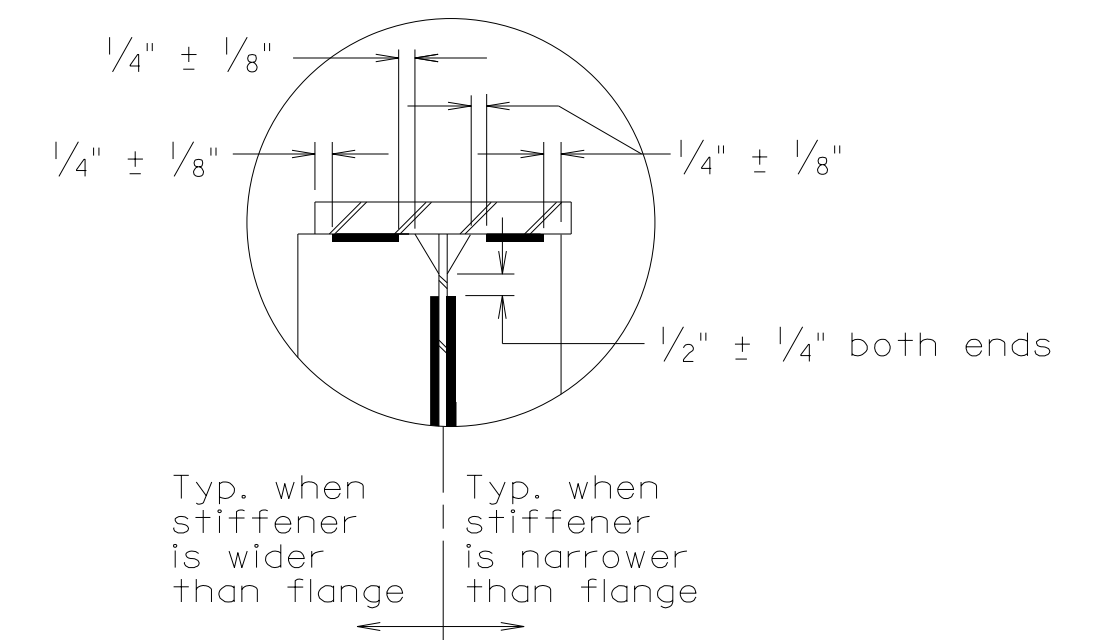
For spacing of intermediate diaphragm connector plates and transverse stiffener plates, see Framing Plan, sheet 16.

For stud shear connector spacing in vicinity of bolted field splice, see Bolted Splice Details, sheet 18.

The bottom flange, web, and all splice plates are areas of tensile stress for Charpy V-Notch impact requirements.

If the Contractor chooses to eliminate one or more bolted field splice(s), the Contractor shall submit their shipping and erection plans along with the shop drawings to the Department for review and approval. The shipping and erection plans shall be signed and sealed by a Professional Engineer, holding a valid license to practice engineering in the Commonwealth of Virginia. Approval for eliminating a bolted field splice does not imply issuance of a hauling permit.

Symbol  $\phi$  = diameter

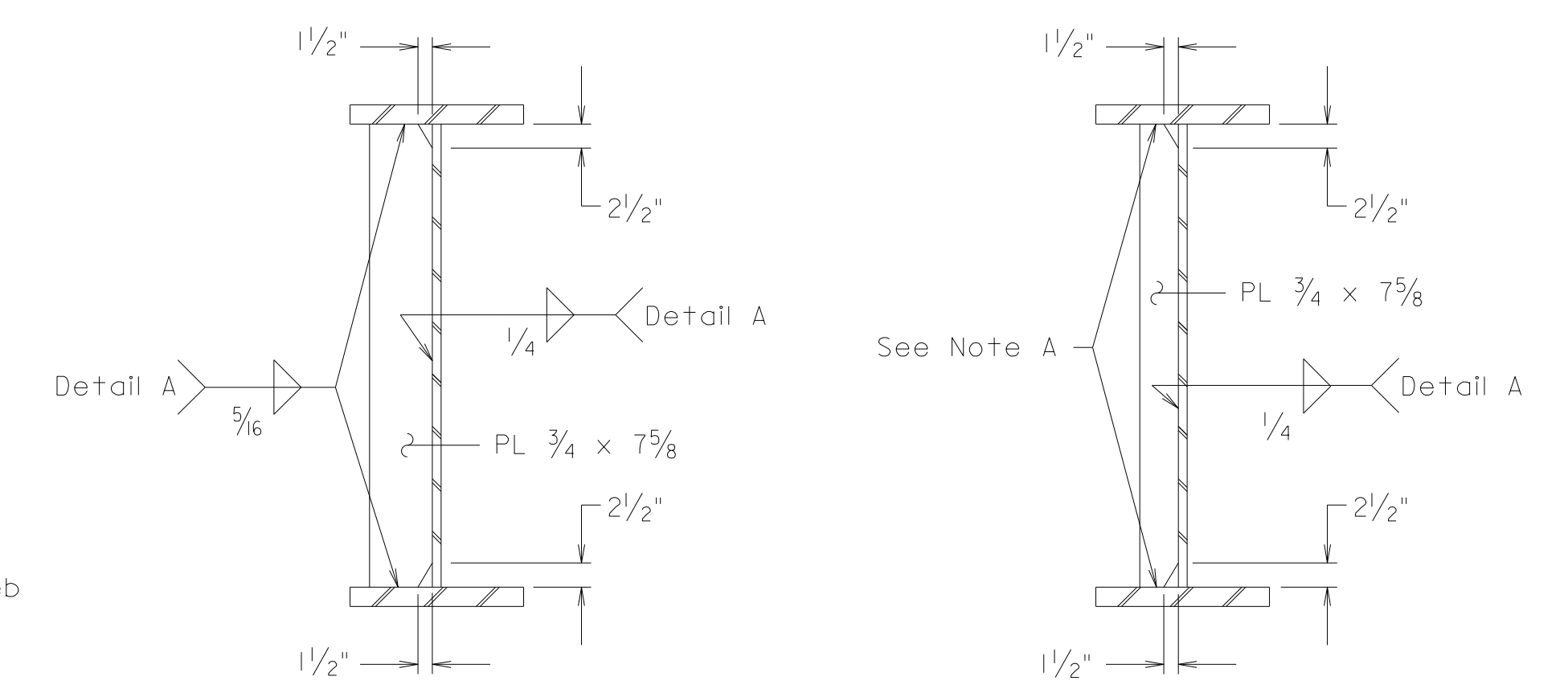


DETAIL A

PLATE DIMENSION TABLE					
Girder	Web PL	PL 1	PL 2	PL 3	PL 4
All	3/16" x 72"	7/8" x 18"	1 3/8" x 18"	1 7/8" x 18"	1 7/8" x 18"

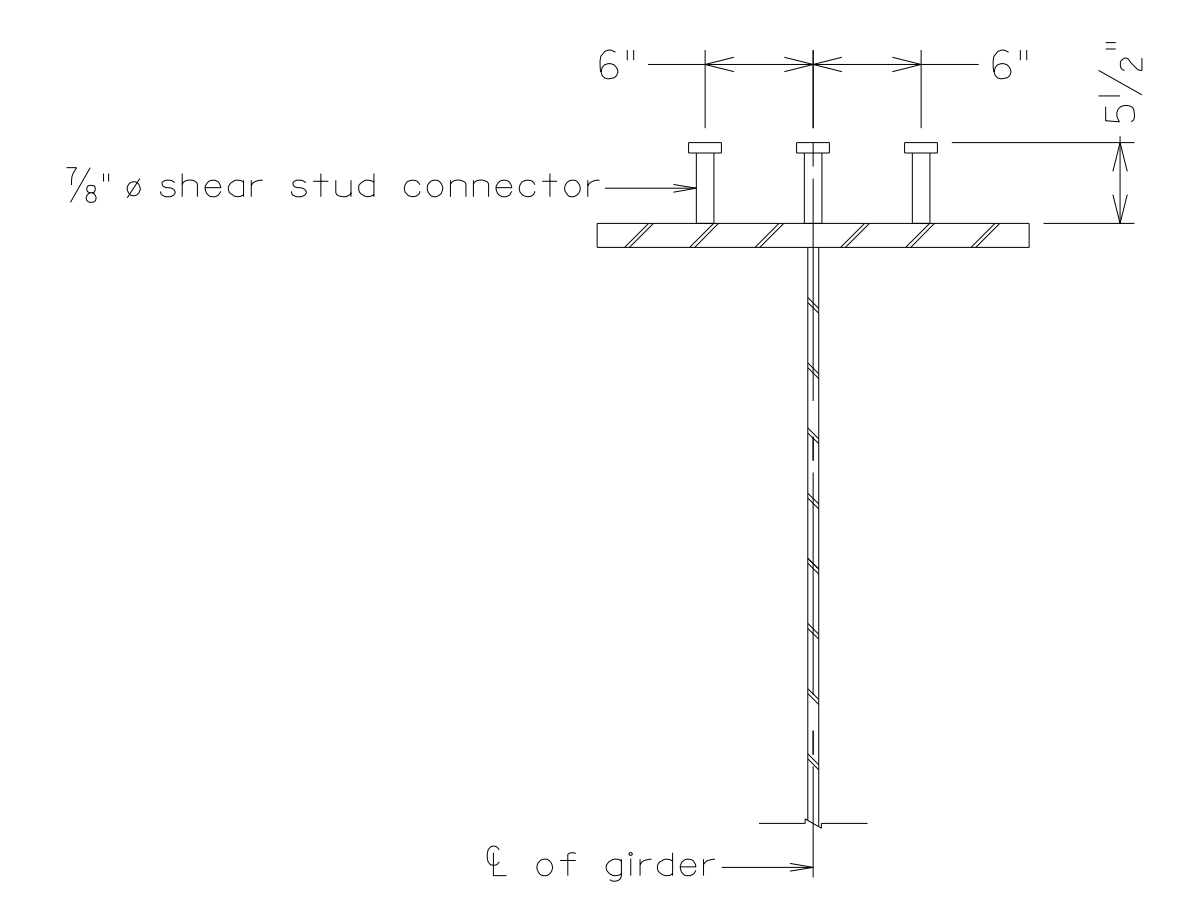
GIRDER DIMENSION TABLE																
Girder	AB	AT	BB	BT	FA	FB	FC	FD	GA	GB	GC	GD	GE	GF	GH	GI
All	51'-10"	51'-10"	70'-0"	70'-0"	4"	173'-0"	4"	40'-0"	3"	168	1'-0"					3"

TENSION FLANGES	
Girder	TI
All	173'-8"

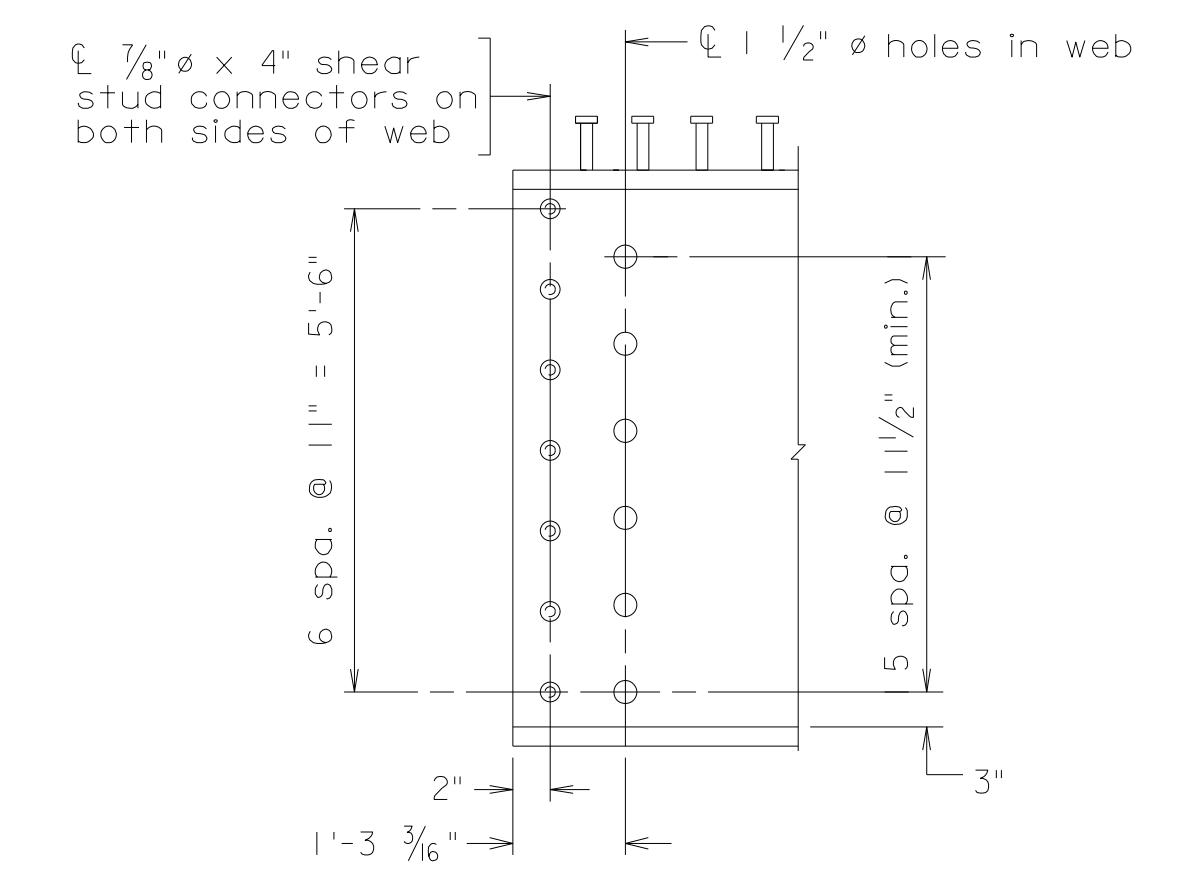


CROSS FRAME CONNECTOR PLATE

TRANSVERSE INTERMEDIATE STIFFENER



TYPICAL SHEAR STUD CONNECTOR DETAIL



TYPICAL BEAM END DETAIL  
(Bearing stiffeners not shown for clarity)

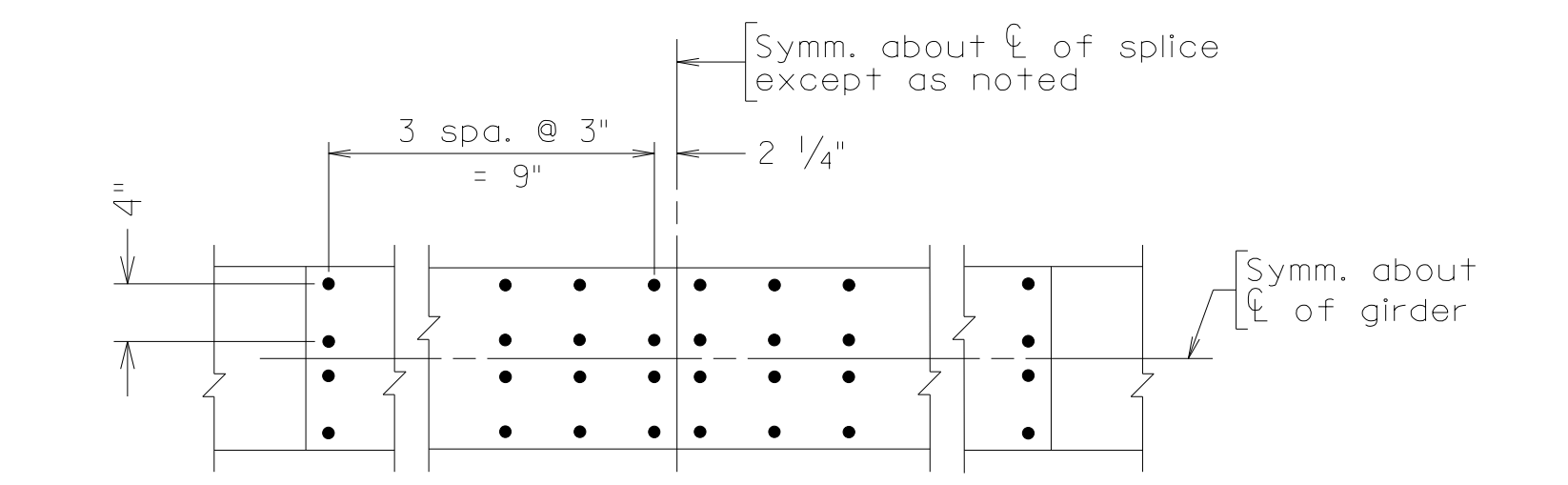
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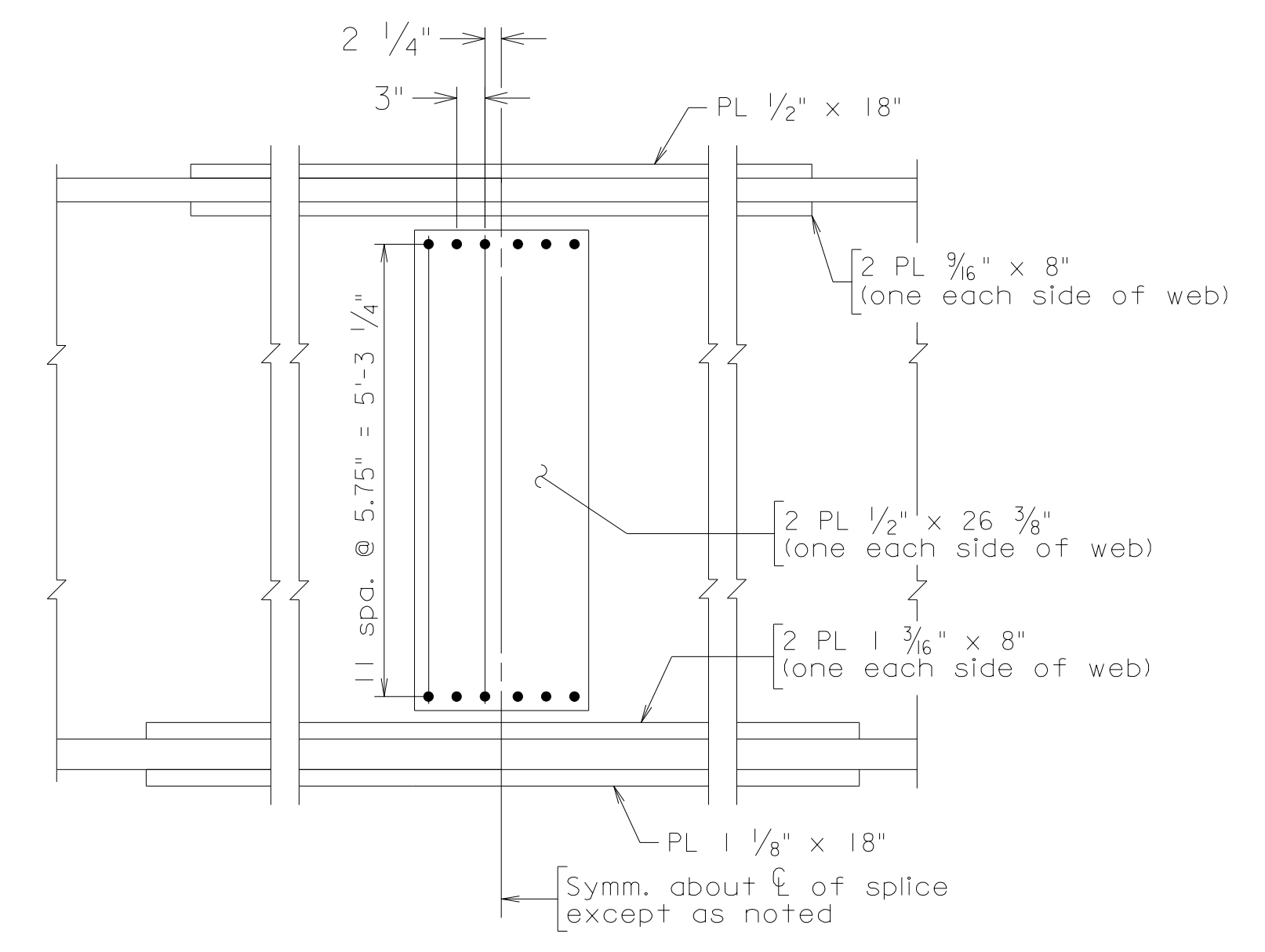
CITY OF CHARLOTTESVILLE			
STRUCTURE AND BRIDGE DIVISION			
GIRDER DETAILS			
No.	Description	Date	Designed: KMR Drawn: DAM Checked: GSC
Revisions		Date	Plan No.
		January 2026	Sheet No.
			17 of 30

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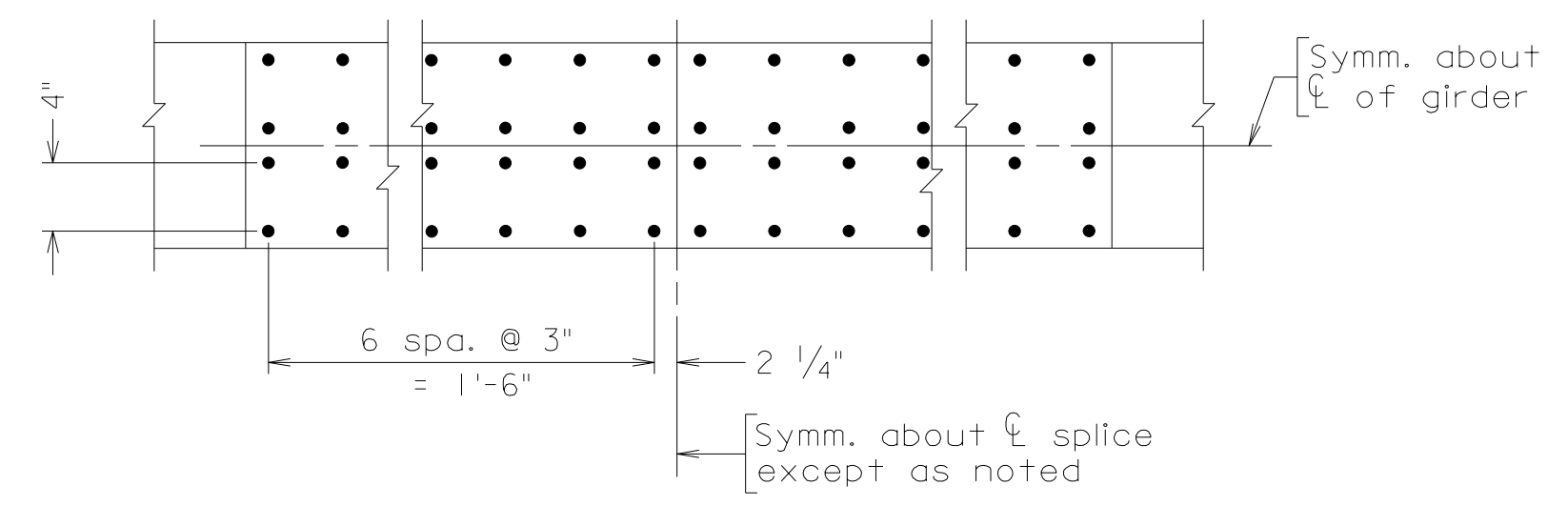
STATE	FEDERAL AID		STATE		SHEET
ROUTE	PROJECT		ROUTE	PROJECT	NO.
VA.	STP-5104 (326)		—	U000-104-365, B620, C501, P101	—



TOP FLANGE



WEB



BOTTOM FLANGE  
BOLTED SPLICE DETAILS

Notes:

Stud shear connectors on top flange not shown. For spacing, see girder details, sheet 17.  
If Contractor increases the girder web thickness in order to eliminate the transverse stiffeners, no change will be made in the web splice.

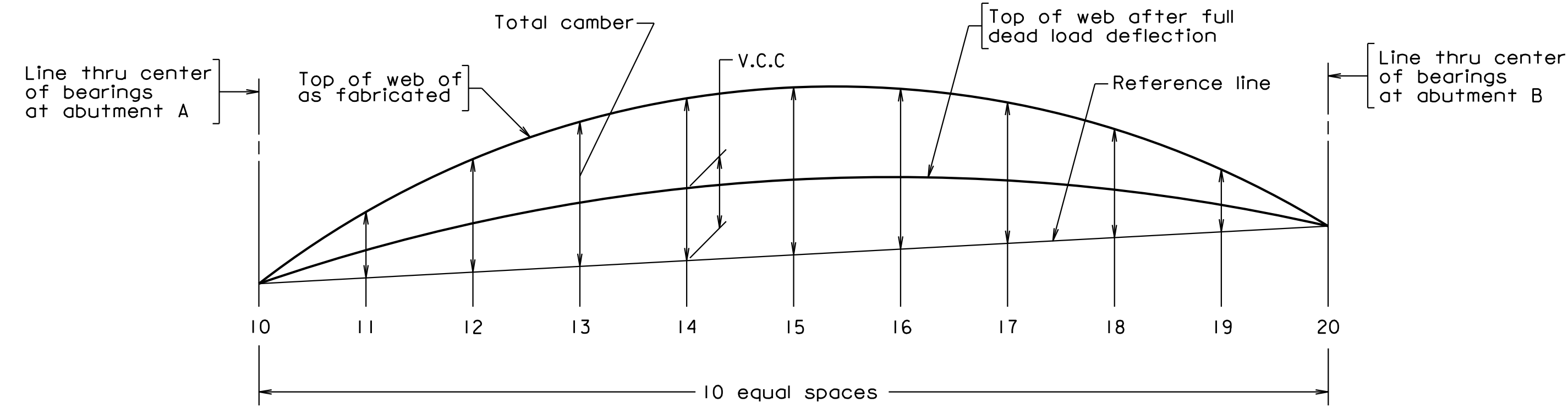
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		CITY OF CHARLOTTESVILLE	
		STRUCTURE AND BRIDGE DIVISION	
		BOLTED SPLICE DETAILS	
No.	Description	Date	Designed: KMR Drawn: DAM Checked: GSC
Revisions		Date	Plan No.
		January 2026	Sheet No.
			18 of 30



	Point	10	11	12	13	14	15	16	17	18	19	20
Girder 1	$\Delta_s$	0.00	0.88	1.65	2.23	2.59	2.71	2.59	2.23	1.65	0.88	0.00
	$\Delta'_s$	0.00	2.52	4.73	6.41	7.44	7.79	7.44	6.41	4.73	2.52	0.00
	$\Delta_c$	0.00	0.25	0.47	0.64	0.75	0.79	0.75	0.64	0.47	0.25	0.00
	V.C.C.	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	Total	0.00	3.64	6.85	9.28	10.77	11.29	10.77	9.28	6.85	3.64	0.00
Girder 2	$\Delta_s$	0.00	0.88	1.65	2.23	2.59	2.71	2.59	2.23	1.65	0.88	0.00
	$\Delta'_s$	0.00	2.53	4.75	6.42	7.45	7.80	7.45	6.42	4.75	2.53	0.00
	$\Delta_c$	0.00	0.25	0.47	0.64	0.75	0.78	0.75	0.64	0.47	0.25	0.00
	V.C.C.	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	Total	0.00	3.65	6.86	9.29	10.79	11.30	10.79	9.30	6.86	3.65	0.00
Girder 3	$\Delta_s$	0.00	0.88	1.65	2.23	2.59	2.71	2.59	2.23	1.65	0.88	0.00
	$\Delta'_s$	0.00	2.51	4.73	6.40	7.50	7.78	7.43	6.40	4.73	2.52	0.00
	$\Delta_c$	0.00	0.25	0.47	0.64	0.75	0.79	0.75	0.64	0.47	0.25	0.00
	V.C.C.	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	Total	0.00	3.64	6.84	9.27	10.77	11.28	10.77	9.27	6.84	3.64	0.00
Girder 4	$\Delta_s$	0.00	0.88	1.65	2.23	2.59	2.71	2.59	2.23	1.65	0.88	0.00
	$\Delta'_s$	0.00	2.51	4.73	6.40	7.43	7.78	7.43	6.40	4.78	2.52	0.00
	$\Delta_c$	0.00	0.25	0.47	0.64	0.75	0.79	0.75	0.64	0.47	0.25	0.00
	V.C.C.	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	Total	0.00	3.64	6.84	9.27	10.76	11.28	10.77	9.27	6.84	3.64	0.00

### CAMBER DIAGRAM

Reference line = line between top of web at abutment A and top of web at abutment B ( $\zeta$  bearing to  $\zeta$  bearing).

$\Delta_s$  = Deflection of girder from its own weight after erection including diaphragms, connectors, etc.

$\Delta'_s$  = Deflection of girder from dead load of concrete deck slab, bolster and construction tolerance.

$\Delta_c$  = Deflection of girder from permanent dead load added after deck slab is cast (e.g. parapet).

V.C.C. = Vertical curve camber = Distance between the reference line and top of web after full dead load deflection.

Total camber =  $\Delta_s + \Delta'_s + \Delta_c + V.C.C.$

Sign convention: Deflections are positive if downward; negative if upward.

V.C.C. is positive if the top of web after full dead load deflection is above the reference line; and negative if below the reference line.

Total camber is positive if the top of web as fabricated is above the reference line; and negative if below the reference line.

Diagram depicts all values being positive, and is not meant to reflect actual conditions.

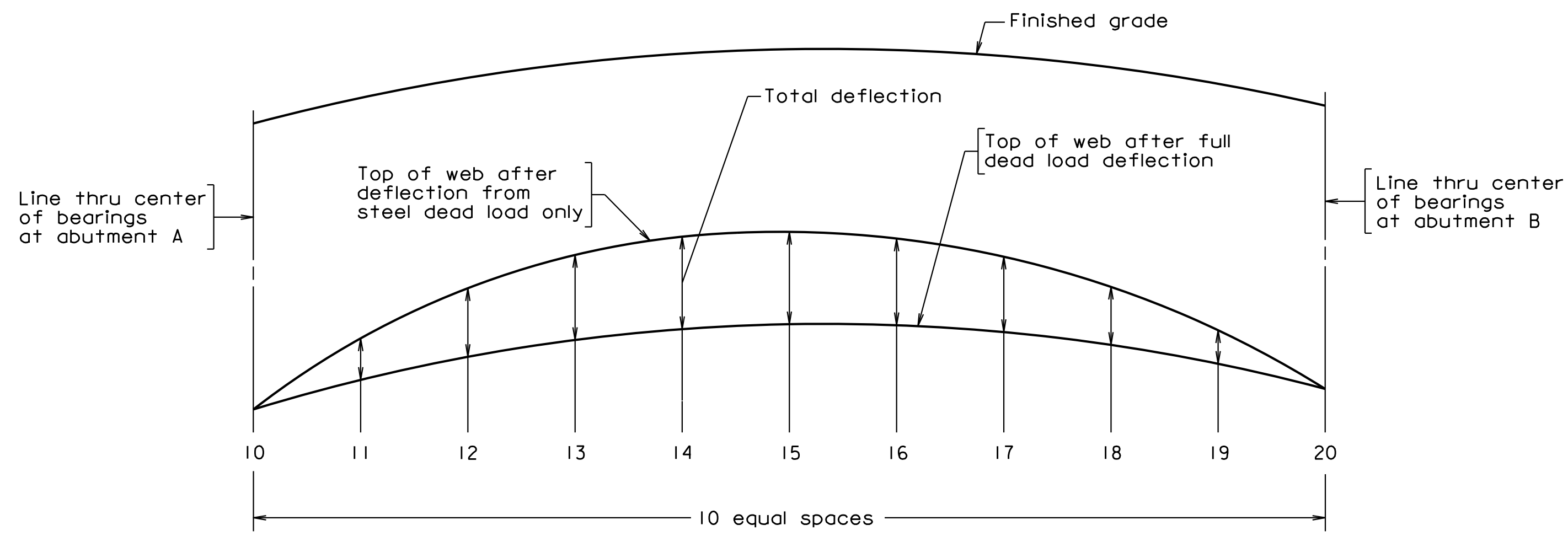
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		CITY OF CHARLOTTESVILLE	
		CAMBER DIAGRAM	
No.	Description	Date	Sheet No.
	Revisions	Designed: KMR Drawn: DAM Checked: GSC	Date: January 2026 Plan No. 19 of 30



	Point	10	11	12	13	14	15	16	17	18	19	20
Girder 1	$\Delta_s$	0.00	2.52	4.73	6.41	7.44	7.79	7.44	6.41	4.73	2.52	0.00
	$\Delta_c$	0.00	0.25	0.47	0.64	0.75	0.79	0.75	0.64	0.47	0.25	0.00
	Total	0.00	2.77	5.20	7.05	8.19	8.58	8.19	7.05	5.20	2.77	0.00
Girder 2	$\Delta_s$	0.00	2.53	4.75	6.42	7.45	7.80	7.45	6.42	4.75	2.53	0.00
	$\Delta_c$	0.00	0.25	0.47	0.64	0.75	0.78	0.75	0.64	0.47	0.25	0.00
	Total	0.00	2.78	5.22	7.06	8.20	8.58	8.20	7.06	5.22	2.78	0.00
Girder 3	$\Delta_s$	0.00	2.51	4.73	6.40	7.43	7.78	7.43	6.40	4.73	2.52	0.00
	$\Delta_c$	0.00	0.25	0.47	0.64	0.75	0.79	0.75	0.64	0.47	0.25	0.00
	Total	0.00	2.76	5.20	7.04	8.18	8.57	8.18	7.04	5.20	2.77	0.00
Girder 4	$\Delta_s$	0.00	2.51	4.73	6.40	7.43	7.78	7.43	6.40	4.73	2.52	0.00
	$\Delta_c$	0.00	0.25	0.47	0.64	0.75	0.79	0.75	0.64	0.47	0.25	0.00
	Total	0.00	2.76	5.20	7.04	8.18	8.57	8.18	7.04	5.20	2.77	0.00

$\Delta_s$  = Deflection of girder from dead load of concrete deck slab, bolster and construction tolerance.  
 $\Delta_c$  = Deflection of girder from permanent dead load added after deck slab is cast (e.g. parapet).

**DEAD LOAD DEFLECTIONS**

TOP OF DECK ELEVATIONS ALONG $\bar{C}$ GIRDER											
Point	10	11	12	13	14	15	16	17	18	19	20
Girder 1	511.18	511.13	511.08	511.03	510.97	510.92	510.87	510.82	510.77	510.72	510.66
Girder 2	511.36	511.30	511.25	511.20	511.15	511.10	511.04	510.99	510.94	510.89	510.84
Girder 3	511.36	511.30	511.25	511.20	511.15	511.10	511.04	510.99	510.94	510.89	510.84
Girder 4	511.18	511.13	511.08	511.03	510.97	510.92	510.87	510.82	510.77	510.72	510.66

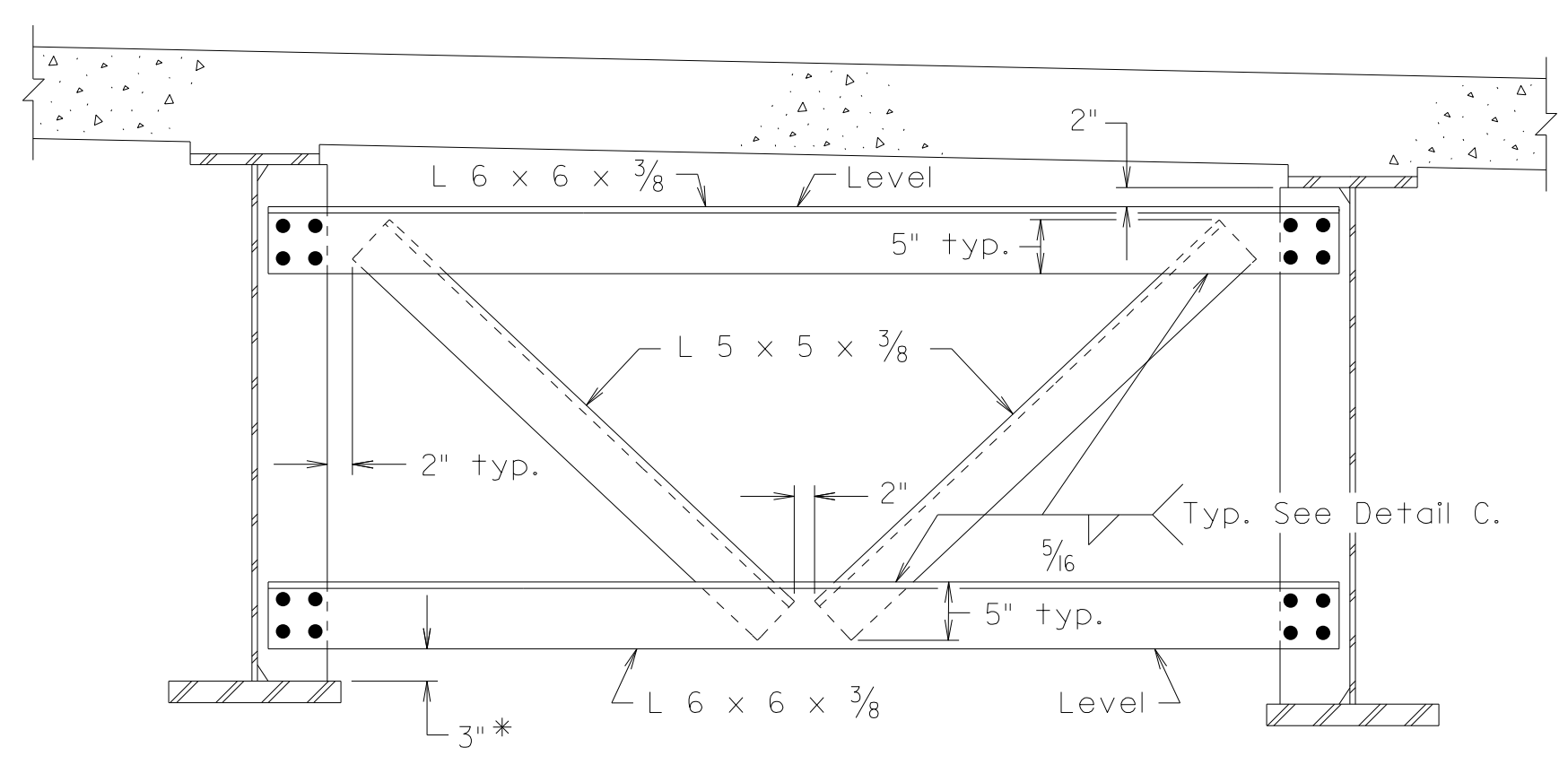
**90% PLANS**  
**THESE PLANS NOT TO BE USED FOR CONSTRUCTION**

KIMLEY-HORN & ASSOC.  
 RICHMOND, VA  
 STRUCTURAL ENGINEER

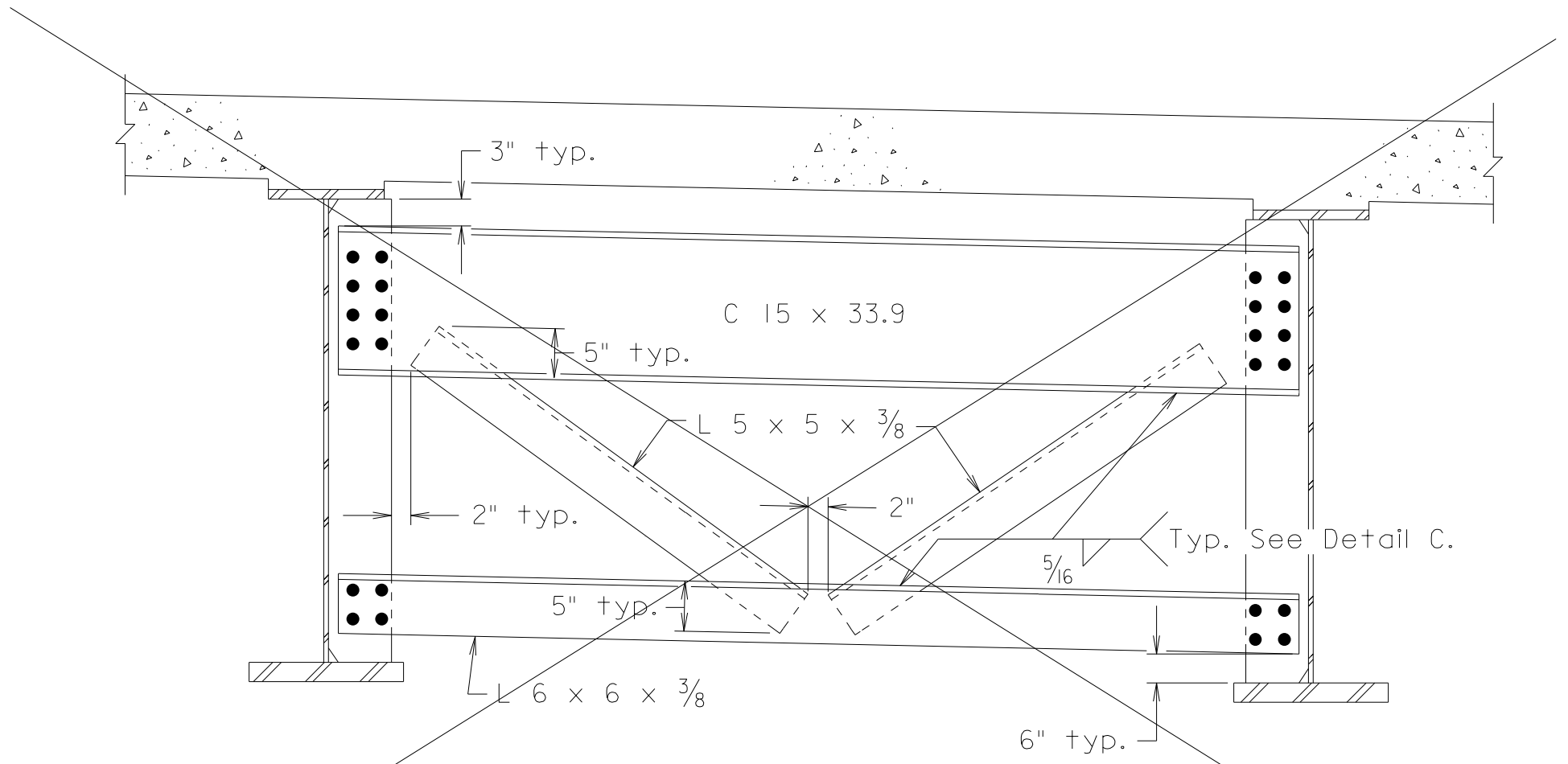
CITY OF CHARLOTTESVILLE					
DEAD LOAD DEFLECTIONS AND SLAB ELEVATIONS					
No.	Description	Date	Designed: KMR	Date	Plan No.
			Drawn: DAM	January	
			Checked: GSC	2026	
Revisions					Sheet No.
					20 of 30

STATE	FEDERAL AID		STATE		SHEET
ROUTE	PROJECT		PROJECT		NO.
VA.	STP-5104 (326)		U000-104-365, B620, C501, P101		--

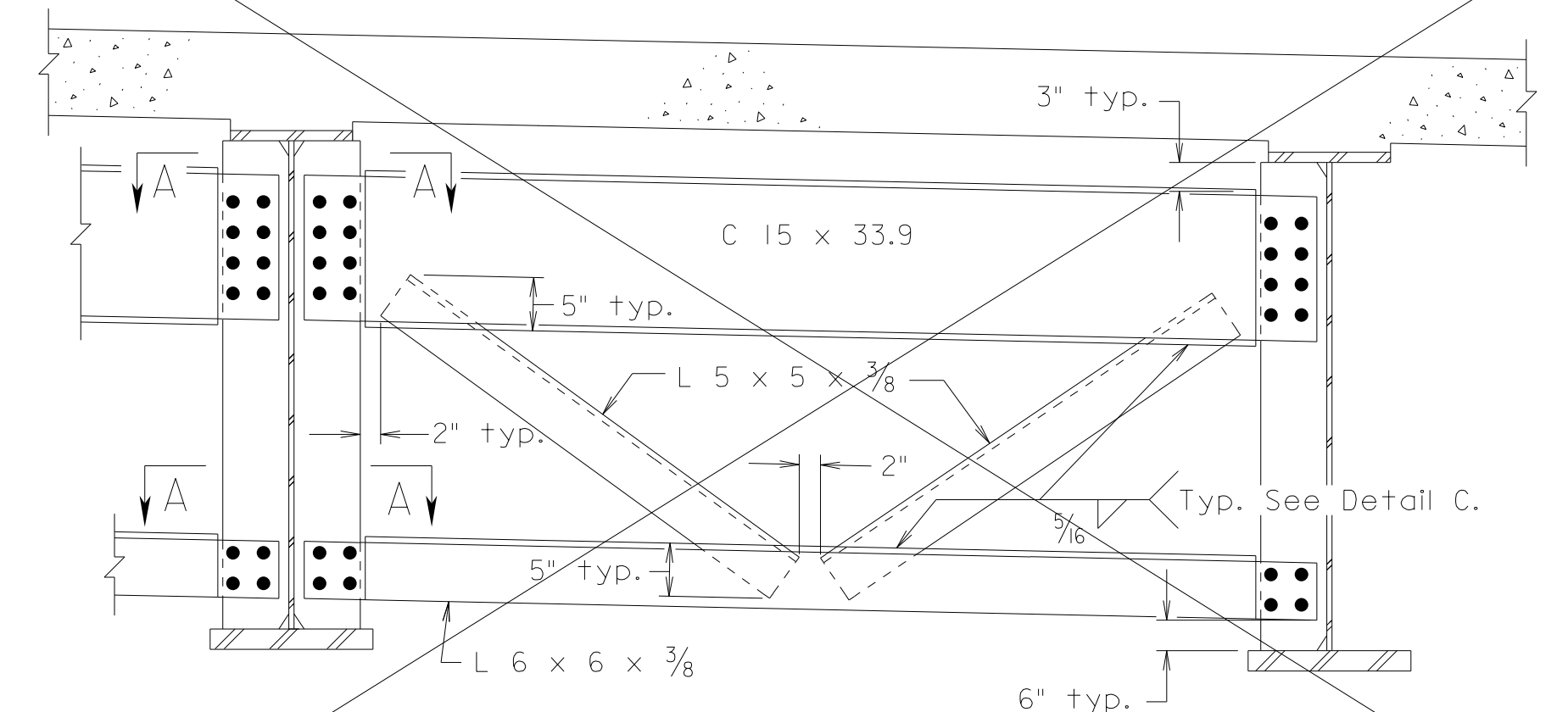
Note:  
 All welding of structural steel and quality control inspection of welds, including field welding and quality control inspection of field welding, shall be the responsibility of the Contractor in accordance with Section 407.04(l) of the Specifications.



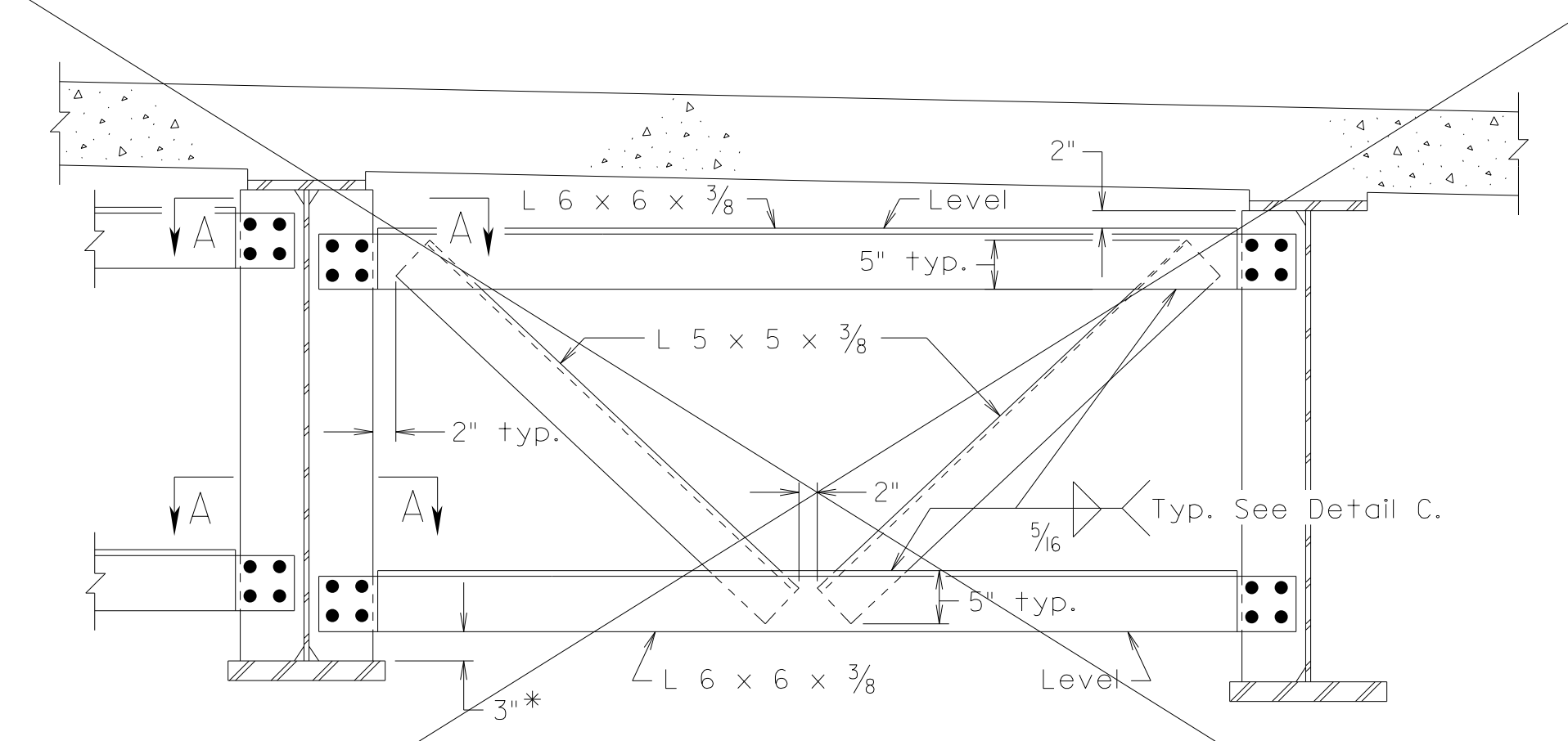
INTERMEDIATE CROSS FRAME - CF1  
 \* Dimension shall be 6" at piers



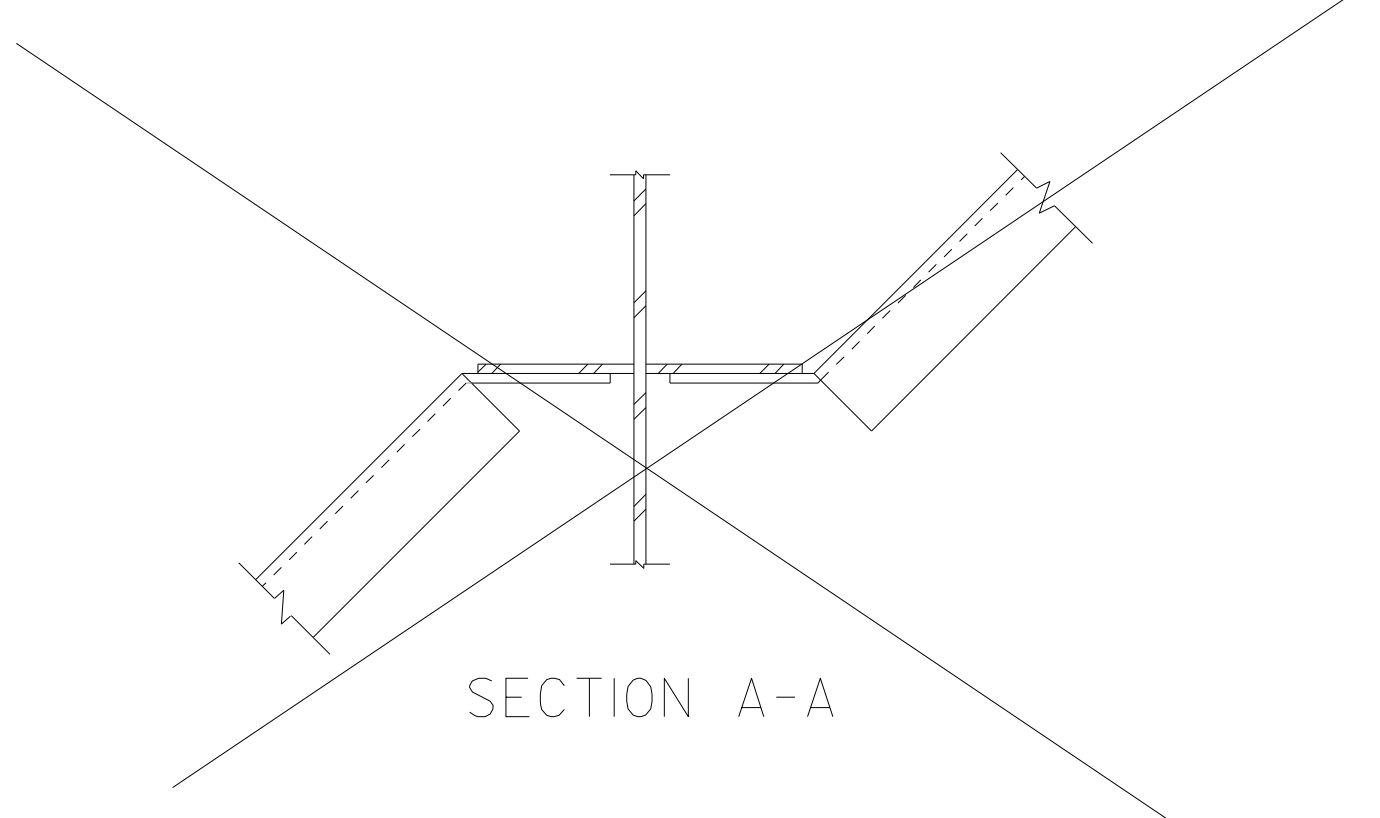
END CROSS FRAME - CF2



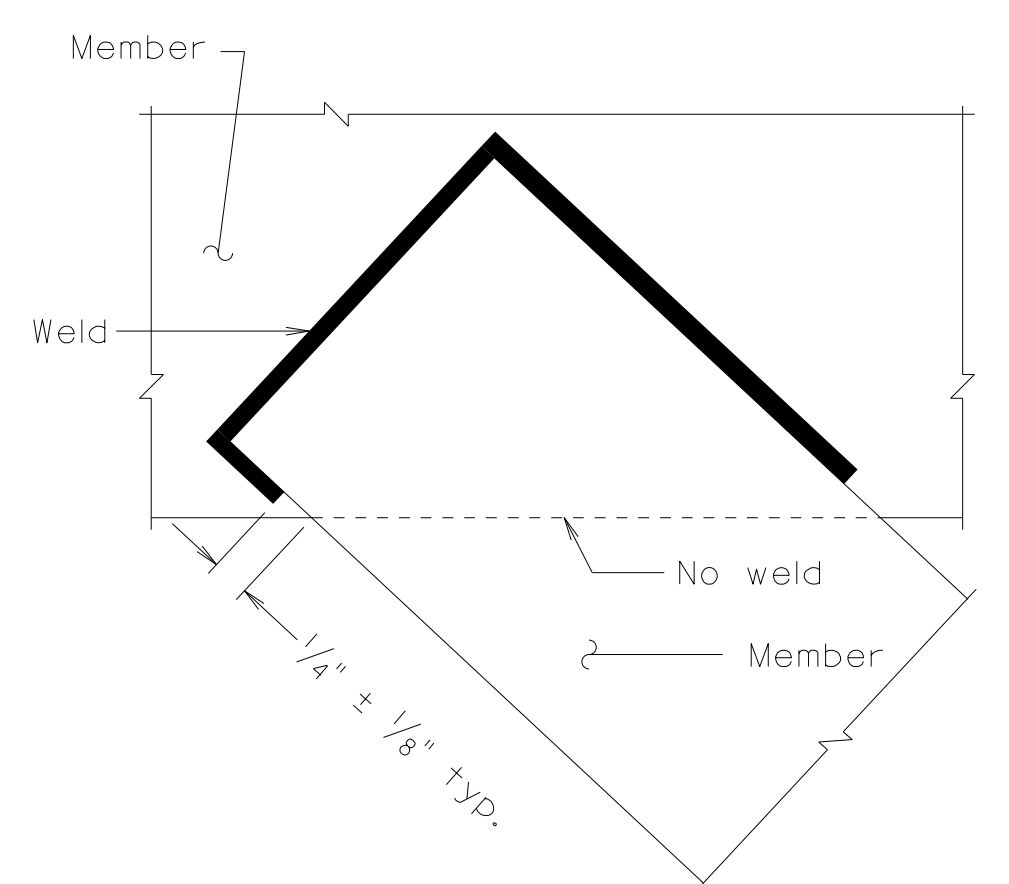
END CROSS FRAME - CF3



INTERMEDIATE CROSS FRAME - CF4  
 \* Dimension shall be 6" at piers



SECTION A-A



DETAIL C

Sealed and Signed by:  
 Junyi Meng  
 Lic. No. 033572  
 On the date of  
 April 30, 2020

A copy of the original  
 sealed and signed  
 standard drawing  
 is on file in the  
 Central Office.

KIMLEY-HORN & ASSOC.  
 RICHMOND, VA  
 STRUCTURAL ENGINEER

90% PLANS  
 THESE PLANS NOT TO BE USED  
 FOR CONSTRUCTION

Not to scale

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CITY OF CHARLOTTESVILLE					
CROSS FRAME DETAILS					
No.	Description	Date	Designed: S&B DIV	Date	Plan No.
			Drawn: S&B DIV	January	
			Checked: S&B DIV	2026	
Revisions					Sheet No.
					21 of 30

STATE	FEDERAL AID		STATE		SHEET
ROUTE	PROJECT		ROUTE	PROJECT	NO.
VA.	STP-5104 (326)		—	U000-104-365, B620, C501, P101	—

Notes:

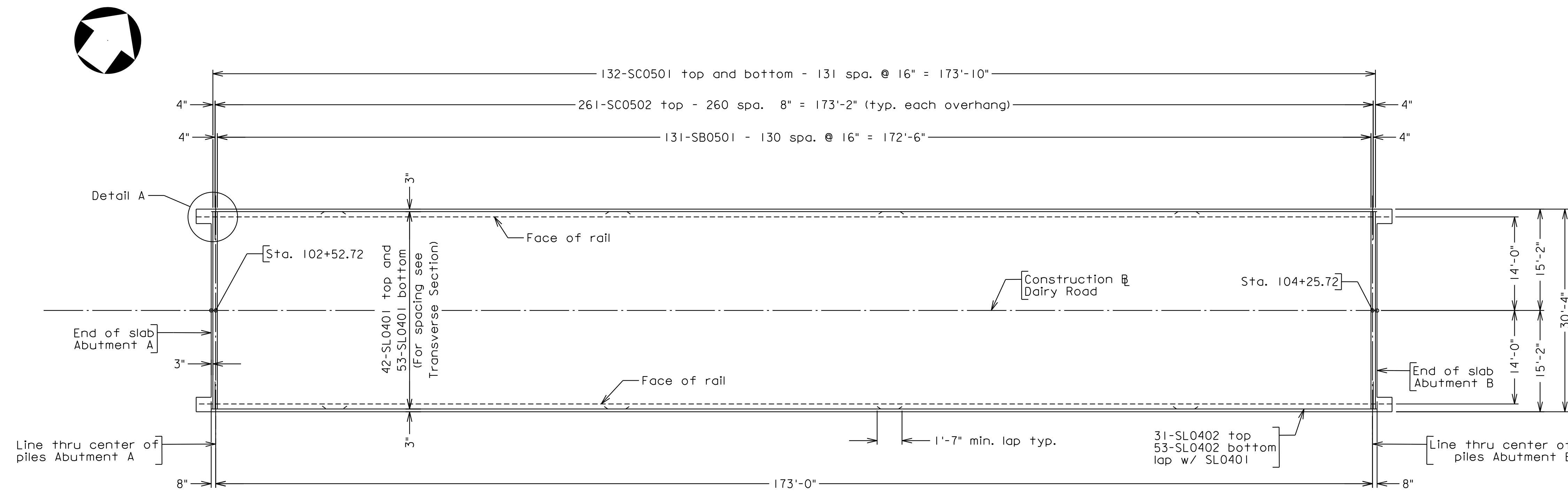
Deck slab elevations are on top of finished roadway at face of rails. Those shown on Dairy Road are at point of finished grade denoted on Transverse Section.

Straight line interpolation for intermediate elevations on top of finished roadway may be made in any direction between any two adjacent points.

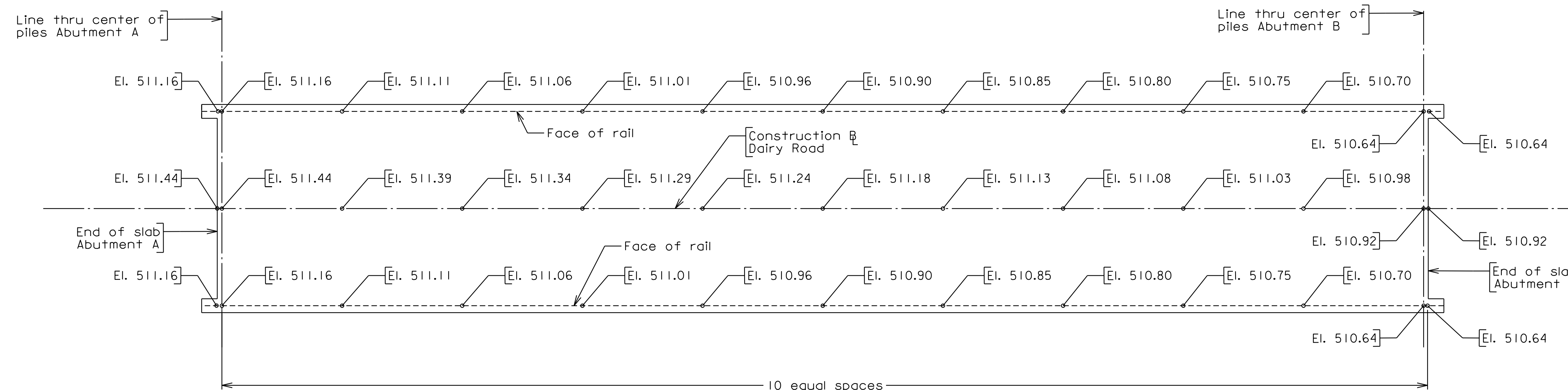
For spacing of SL0401 and SL0402 bars, see sheet 15.

For parapet and terminal wall details, see sheet 23 and 24.

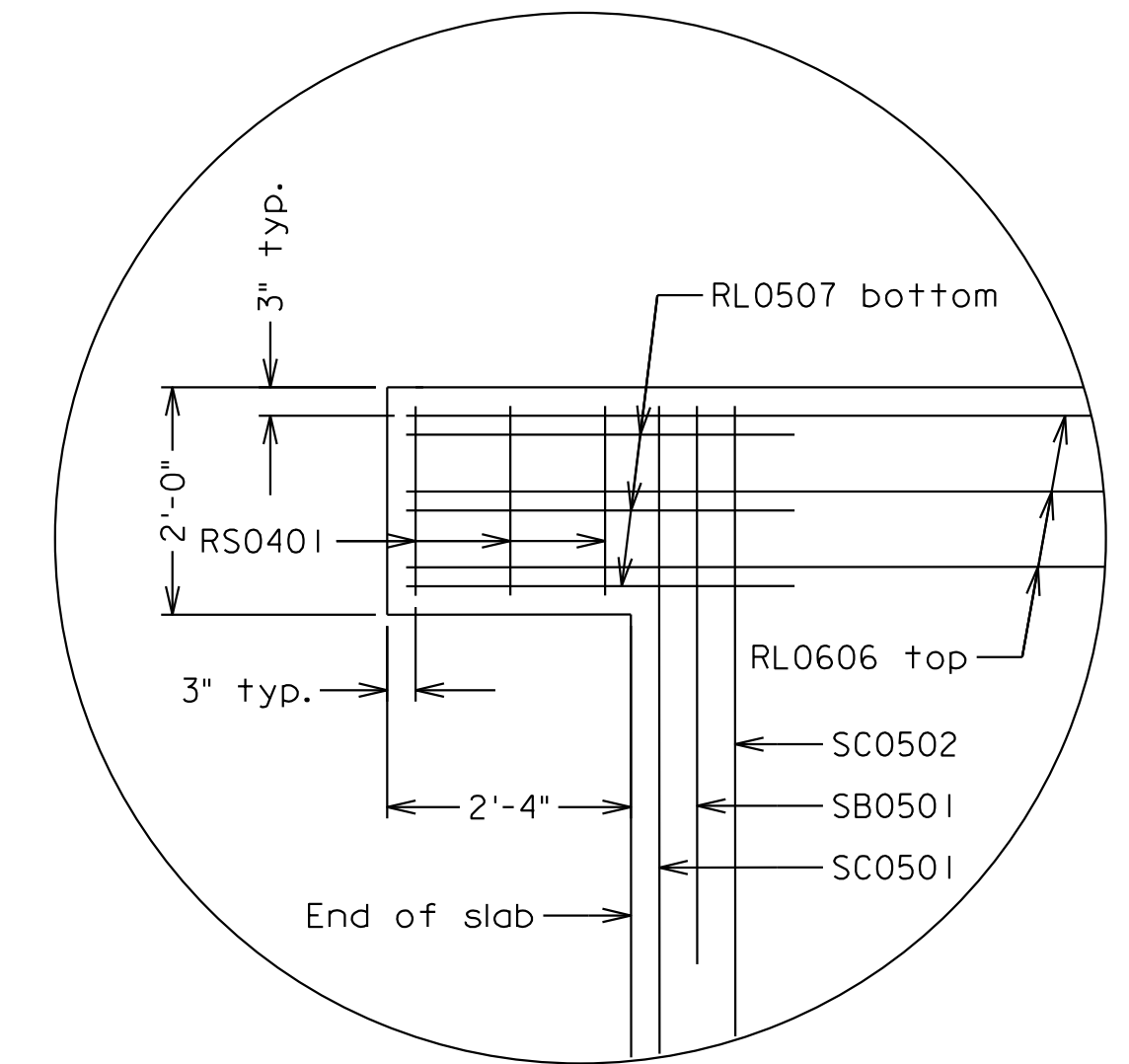
For spacing of RL series and RS0401 bars, see sheet 24.



DECK SLAB PLAN



DECK SLAB ELEVATIONS



DETAIL A

Only deck corner reinforcement shown for clarity. Abutment A shown, Abutment B similar. Not to scale.

**90% PLANS**  
**THESE PLANS NOT TO BE USED FOR CONSTRUCTION**

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 RICHMOND, VA  
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Scale: 1" = 10'-0" © 2026, Commonwealth of Virginia

CITY OF CHARLOTTESVILLE					
DECK PLAN					
No.	Description	Date	Designed: KMR	Date	Plan No.
			Drawn: DAM	January	
			Checked: DGD	2026	
Revisions					Sheet No.
					22 of 30

STATE	FEDERAL AID		STATE		SHEET
VA.	ROUTE	PROJECT	ROUTE	PROJECT	NO.
		STP-5104 (326)		U000-104-365, B620, C501, P101	---

Notes:

Plan dimensions shown are measured in the respective horizontal and vertical planes.

The Contractor shall determine all dimensions and details necessary for installation.

All concrete shall be Low Shrinkage Class A4 Modified.

All levels for concrete shall be 3/4".

All reinforcing steel shall be Corrosion Resistant Reinforcing Steel, Class I.

For details and reinforcing steel schedule of terminal wall, see sheet 20.

Posts and rail members, including pipe sleeves, shall be ASTM A500 Grade B steel. Plates shall be ASTM A36 steel. Charpy V-notch testing is not required for HSS members.

Bolts for attaching rails to posts are 3/4" diameter round head (with slot in head), ASTM A449.

For bolts attaching rails to posts, bolt extensions beyond nut shall be limited to the smaller of one and a half finishing turns or 1/4". If the extension is longer, excess shall be cut off and the edges of the bolt end ground so that no sharp edges remain. Cold galvanizing shall be applied to damaged galvanized areas.

All bolts shall be snug tightened.

All steel, including hardware, shall be hot dip galvanized.

Posts shall be equally spaced within a span. Maximum spacing is 6'-8".

Posts shall be seated on neoprene pads 1/8" minimum thickness, having a nominal durometer hardness of 60. Pads shall conform to post base dimensions.

Posts shall be vertical in transverse direction and normal to longitudinal profile grade. Cut bottom of posts to meet these configurations.

Rails to be continuous over a minimum of 3 posts before splicing.

Bid price for architectural treatment includes concrete in relief and coping.

For additional notes, see sheet for the CPSR-3.

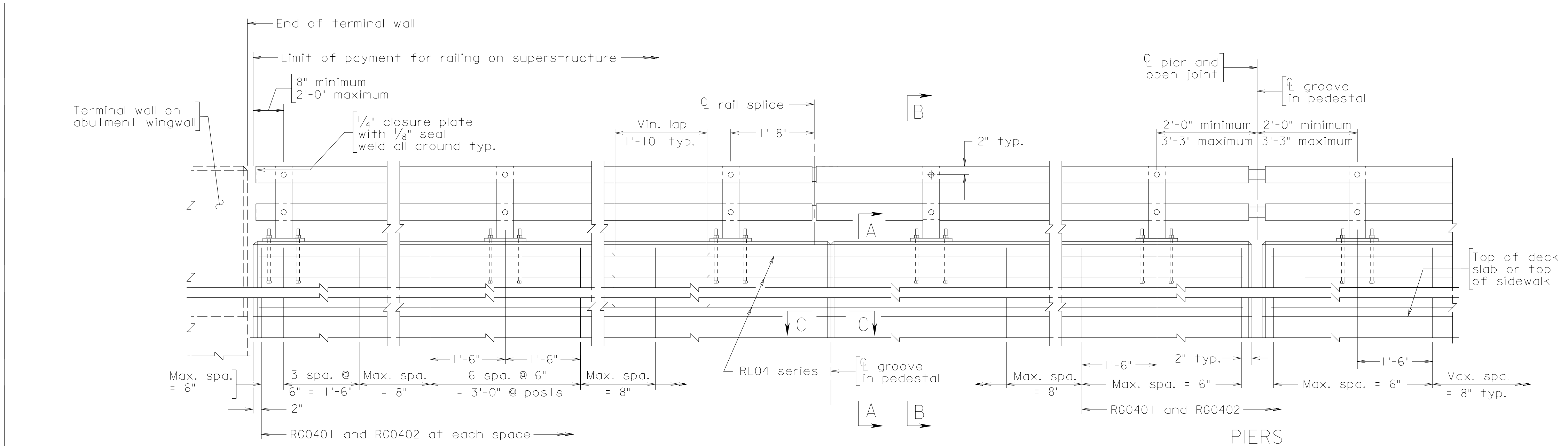
After galvanizing, all exposed surfaces, including all hardware (except hardware attaching the post base plates), shall be powder coated in accordance with the Special Provision for Powder Coated Galvanized Material. Finished color shall be brown, Federal Standard Color No. 595-20059.

After railing installations are completed, galvanized hardware used in the attachment of post base plates shall be field coated using System F. The coating shall match the specified railing color.

REINFORCING STEEL SCHEDULE					
Mark	Size	No.	Length	Pin $\phi$	Location
RG0401	#4		3"		Parapet
RG0402	#4		3"		Parapet
RG0403	#4		3"		Parapet
RG0404	#4		3"		Parapet
RL04	#4				Parapet

Dimensions in bending diagram are out-to-out of bars.

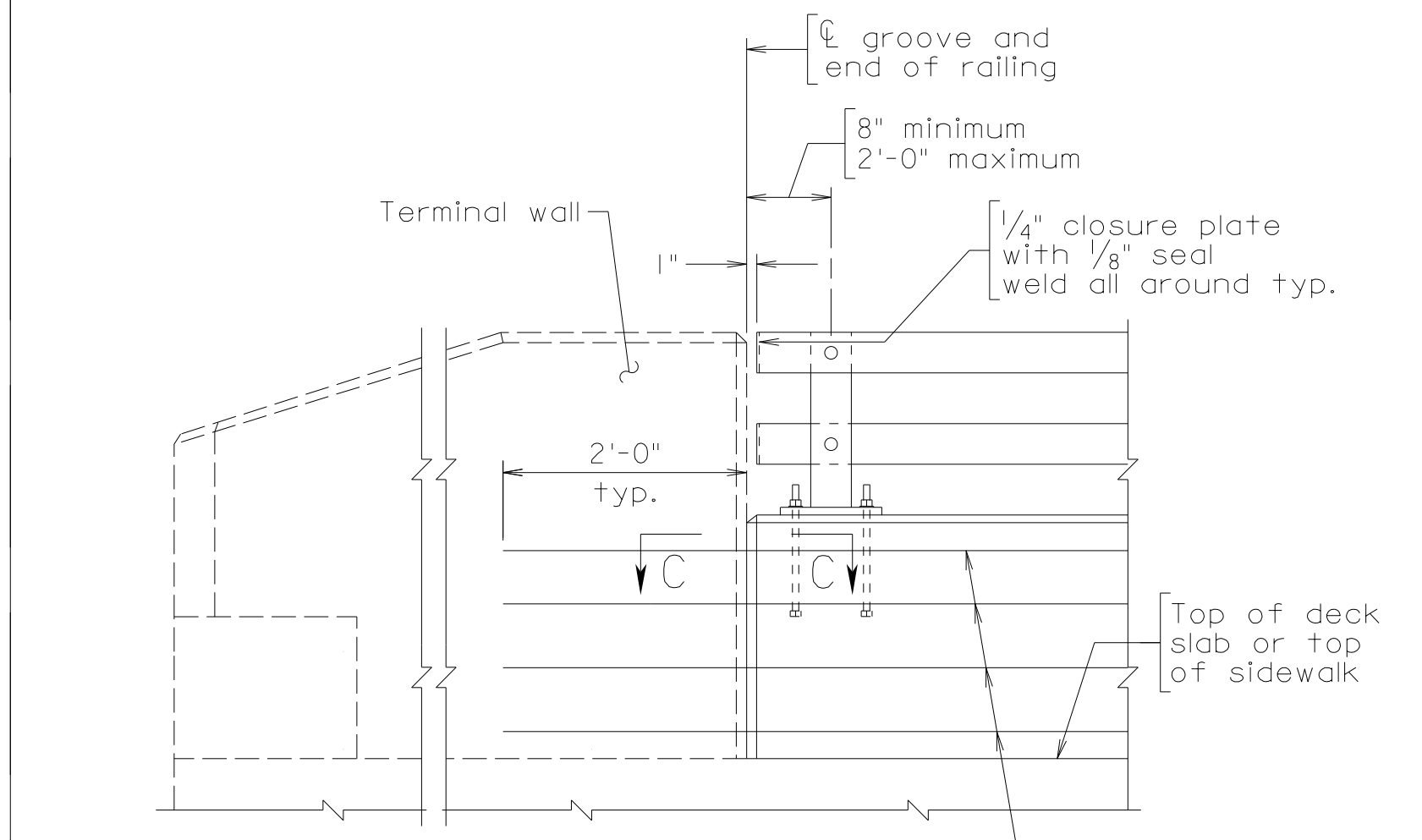
CITY OF CHARLOTTESVILLE					
STRUCTURE AND BRIDGE DIVISION					
42" RAILING CPSR-1-AT					
No.	Description	Date	Designed: S&B DIV	Date	Plan No.
	Revisions		Drawn: S&B DIV	January 2026	
			Checked: S&B DIV		Sheet No.
					23 of 30



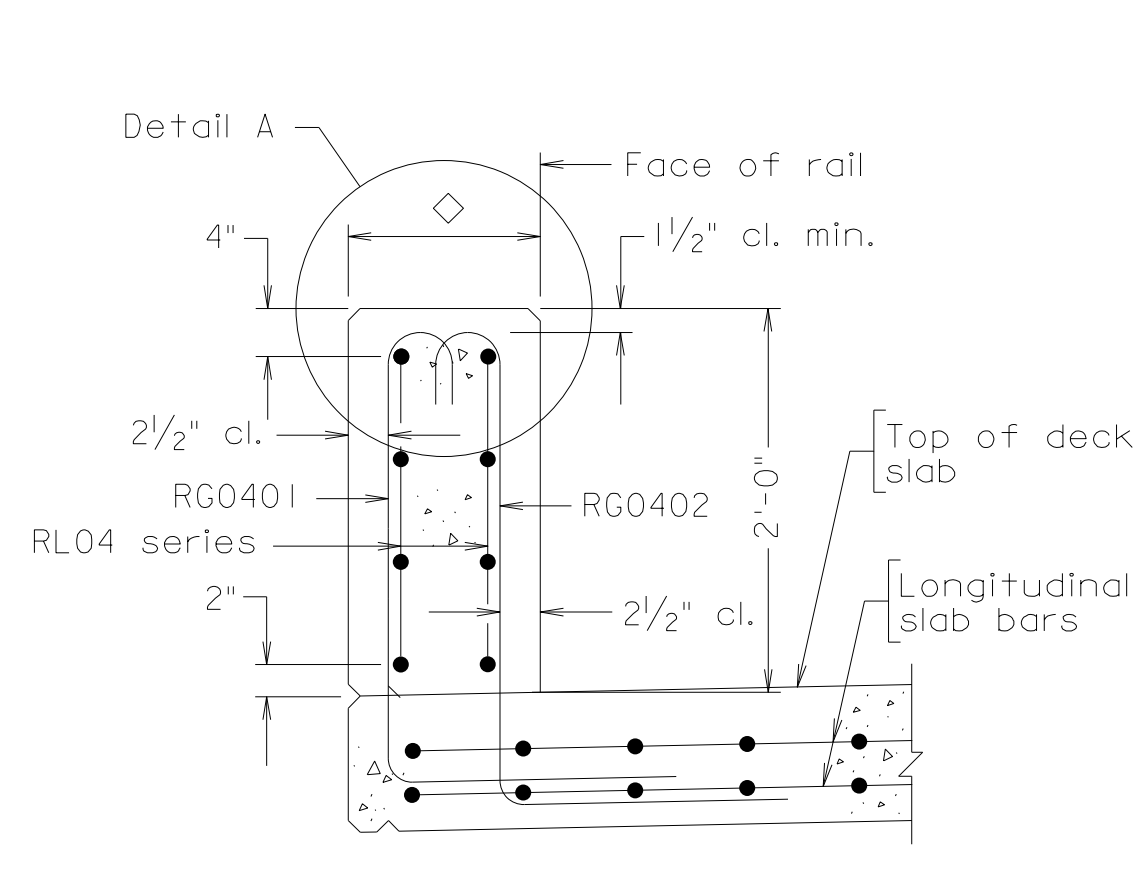
ABUTMENTS  
Terminal Wall on Wingwall

ELEVATION

PIERS  
With joint in deck slab

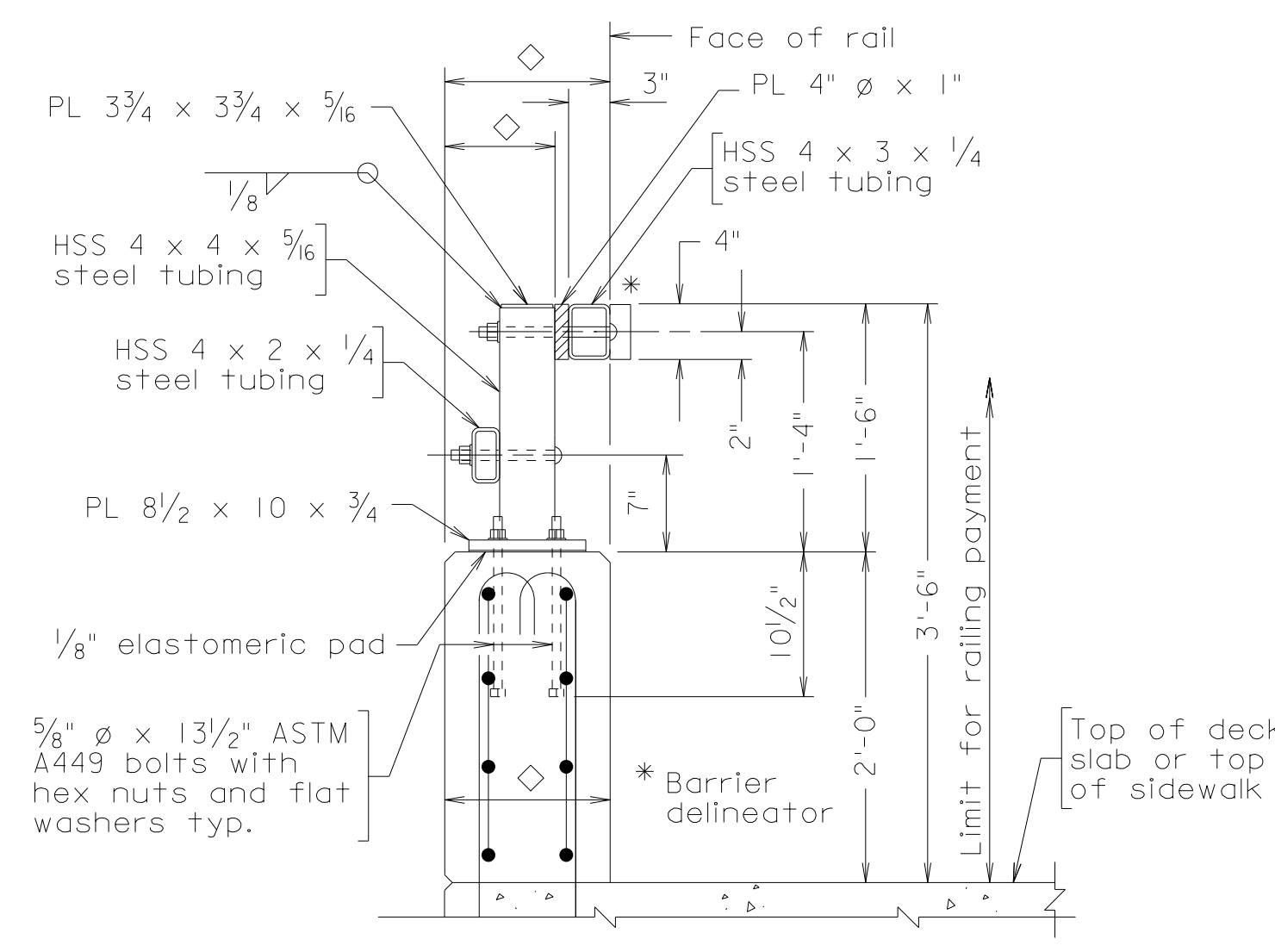


PART ELEVATION  
Terminal Wall on Superstructure



SECTION A-A  
Scale: 1" = 1'-0"

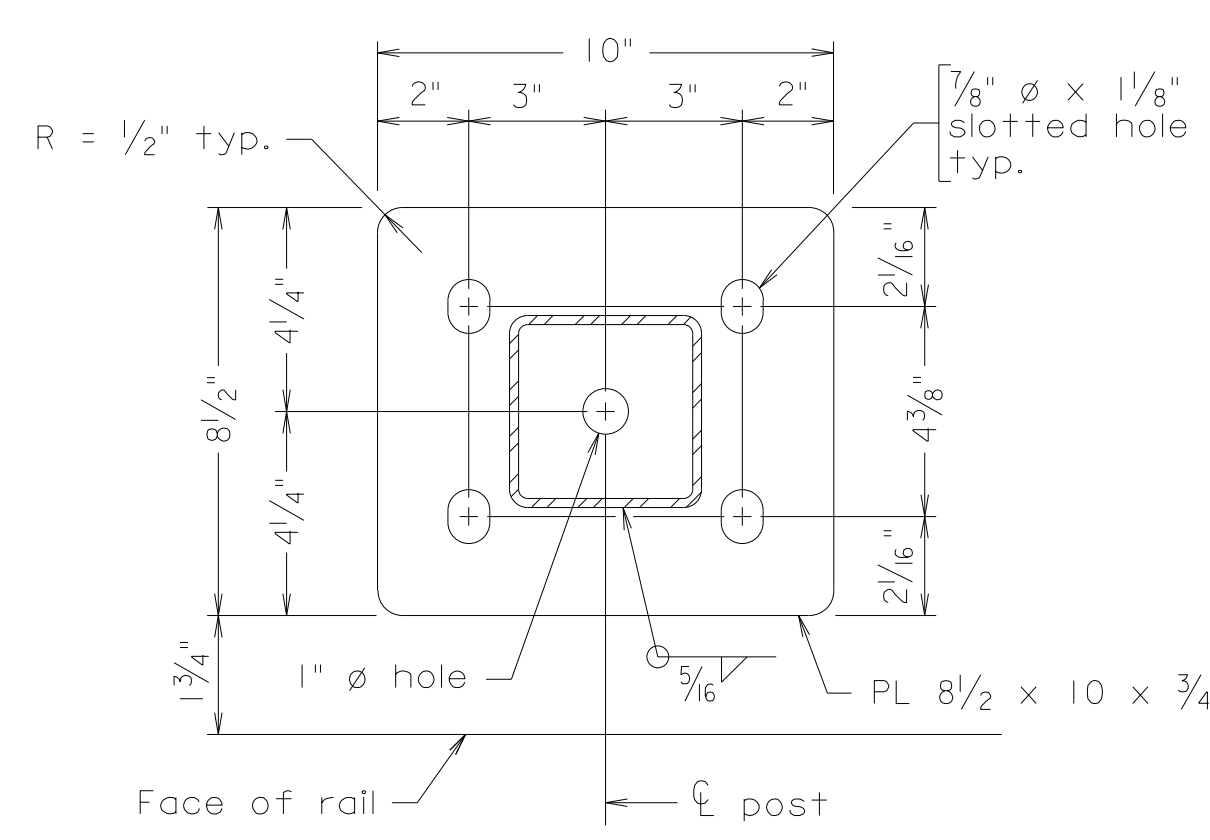
Transverse rebars in deck slab not shown for clarity



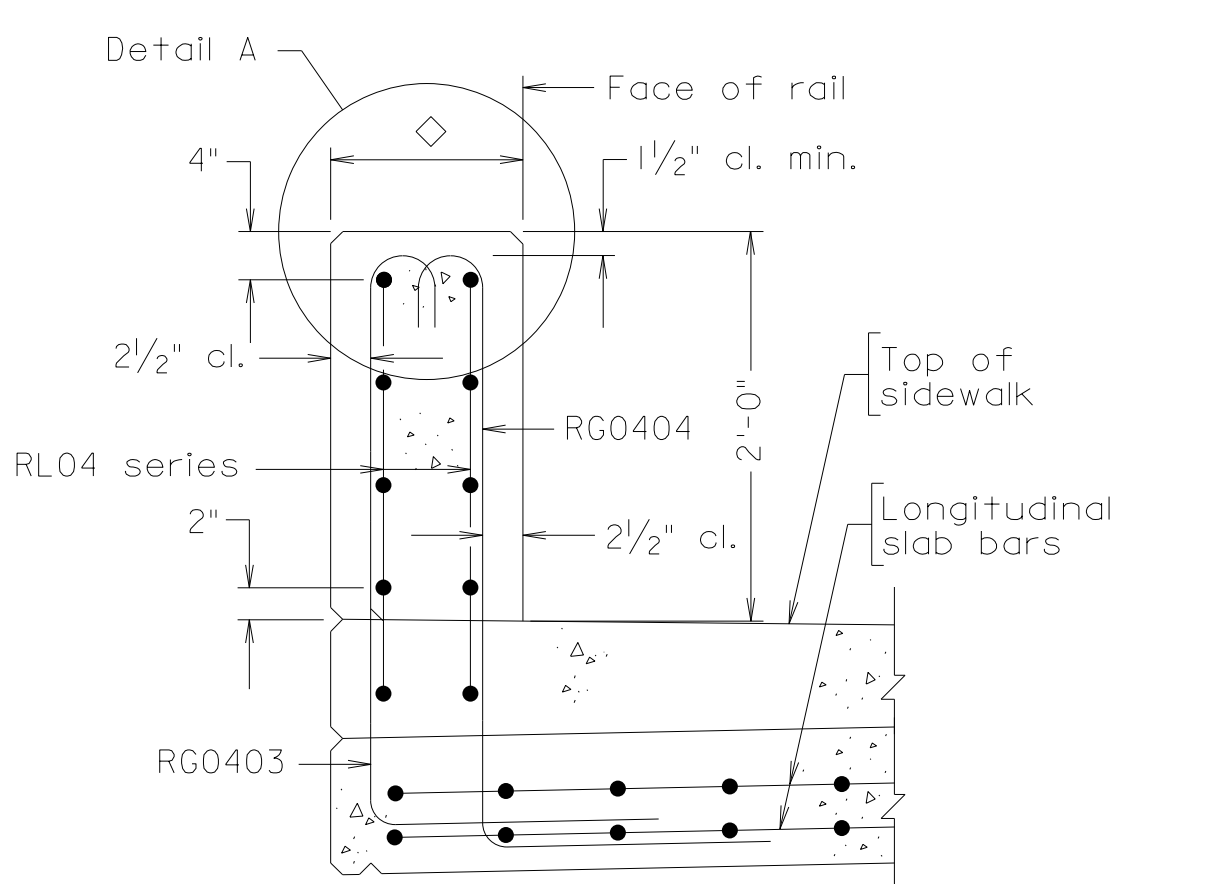
SECTION B-B  
Scale: 1" = 1'-0"

Bolts through base plate shall be contained inside rebar cage

ABUTMENT

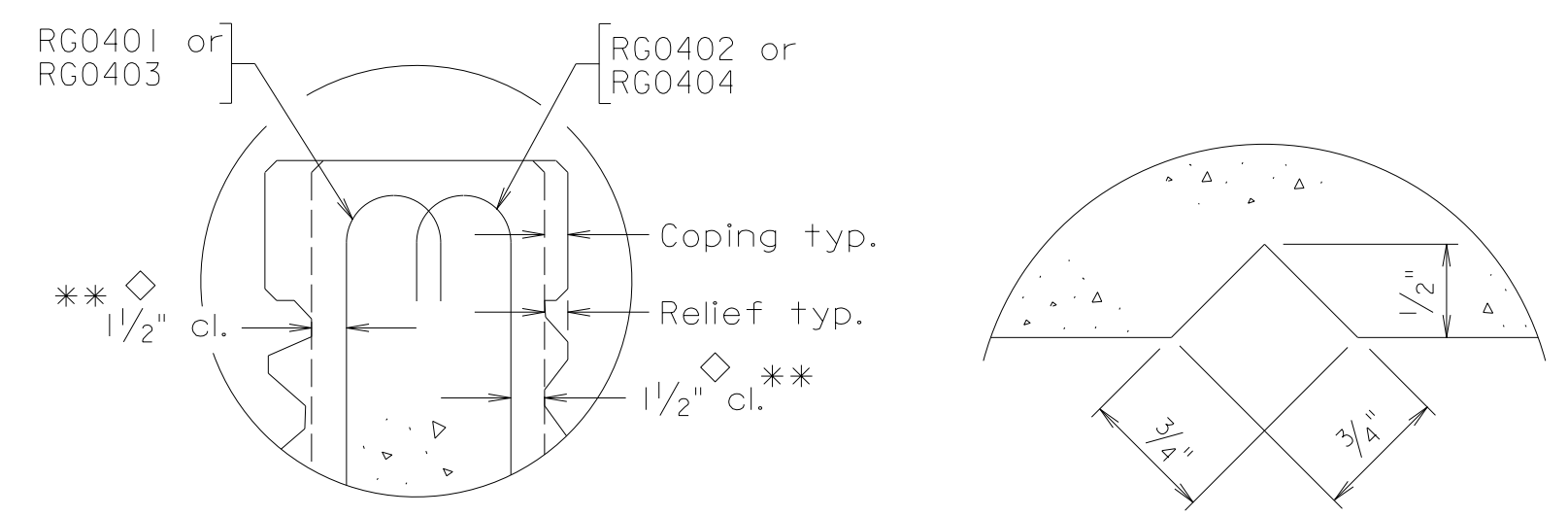


BASE PLATE DETAIL  
Not to scale

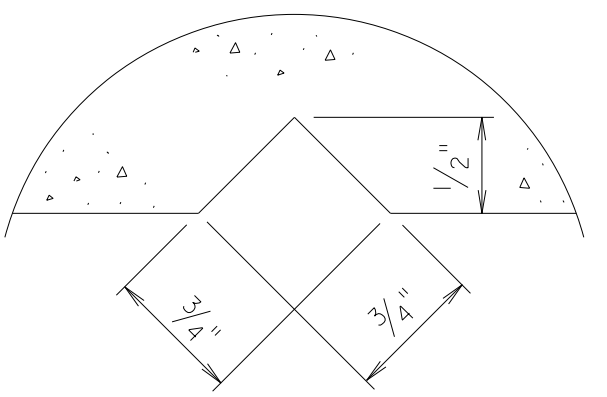


SECTION A-A  
(With sidewalk)  
Not to scale

Reinforcement in sidewalk and transverse rebars in deck slab not shown for clarity



DETAIL A  
Shown with architectural treatment on both sides



SECTION C-C  
Full scale  
Groove detail for both sides of rail

For dimensions and architectural treatment details, see sheet XX. For minimum cover, see Detail A.

\*\* The cover tolerance referenced in the VDOT Road and Bridge Specifications as -0" to +1/2" is shifted to -1/4" to +1/4" for placement of the RG04 series bars.

Scale: 3/4" = 1'-0" unless otherwise noted

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Sealed and Signed by:  
Junyi Meng  
Lic. No. 033572  
On the date of  
October 31, 2023

A copy of the original sealed and signed standard drawing is on file in the Central Office.

KIMLEY-HORN & ASSOC.  
RICHMOND, VA  
STRUCTURAL ENGINEER

90% PLANS  
THESE PLANS NOT TO BE USED FOR CONSTRUCTION

STATE	FEDERAL AID		STATE		SHEET NO.
VA.	ROUTE	PROJECT	ROUTE	PROJECT	---
		STP-5104 (326)		U000-104-365, B620, C501, P101	

Notes:

Plan dimensions shown are measured in the respective horizontal and vertical planes.

The Contractor shall determine all dimensions and details necessary for installation.

All concrete shall be Low Shrinkage Class A4 Modified.

All bevels for concrete shall be 3/4".

All reinforcing steel shall be Corrosion Resistant Reinforcing Steel, Class 1.

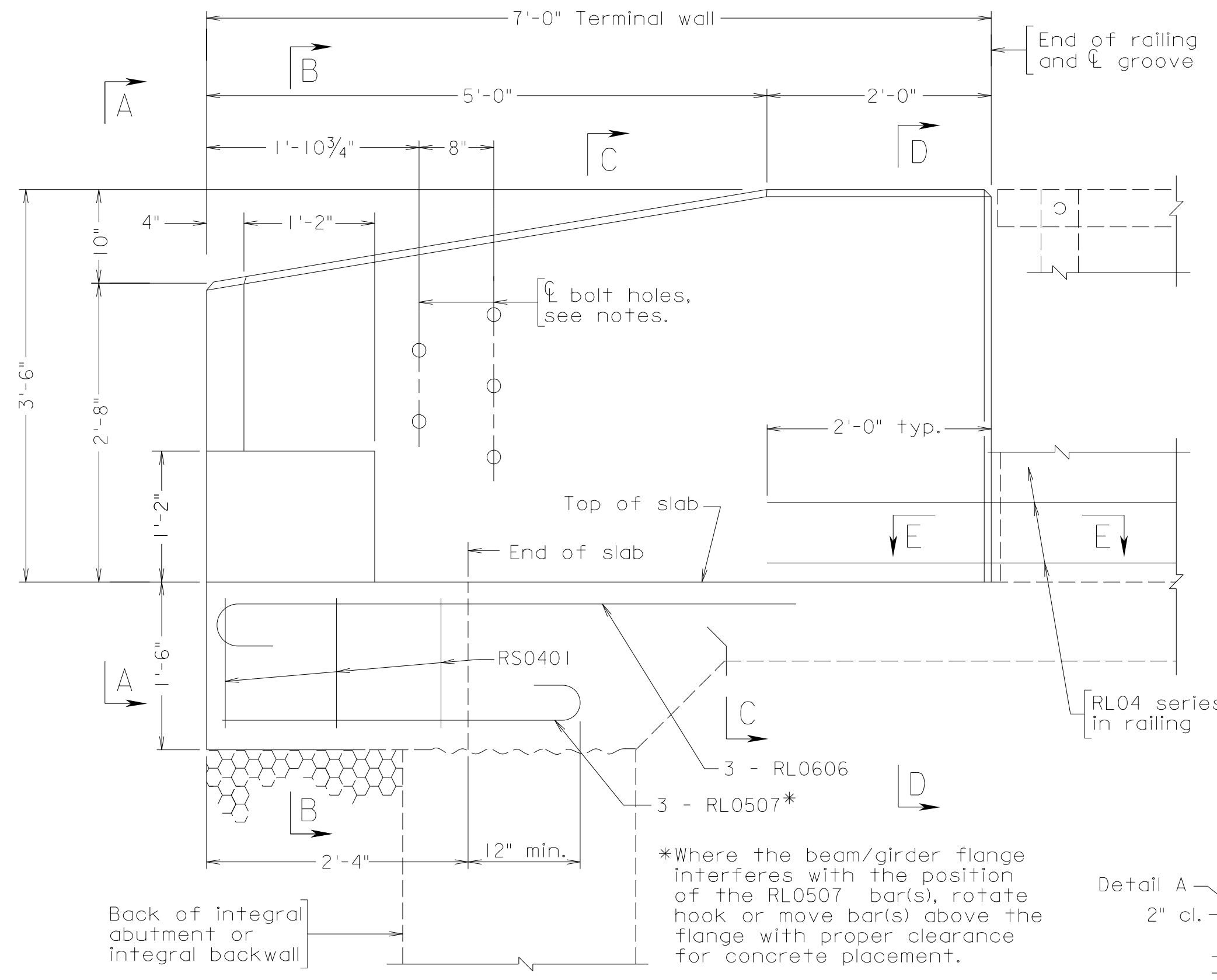
For details and reinforcing steel schedule of railing, see sheet 19.

Each terminal wall shall be cast as one piece.

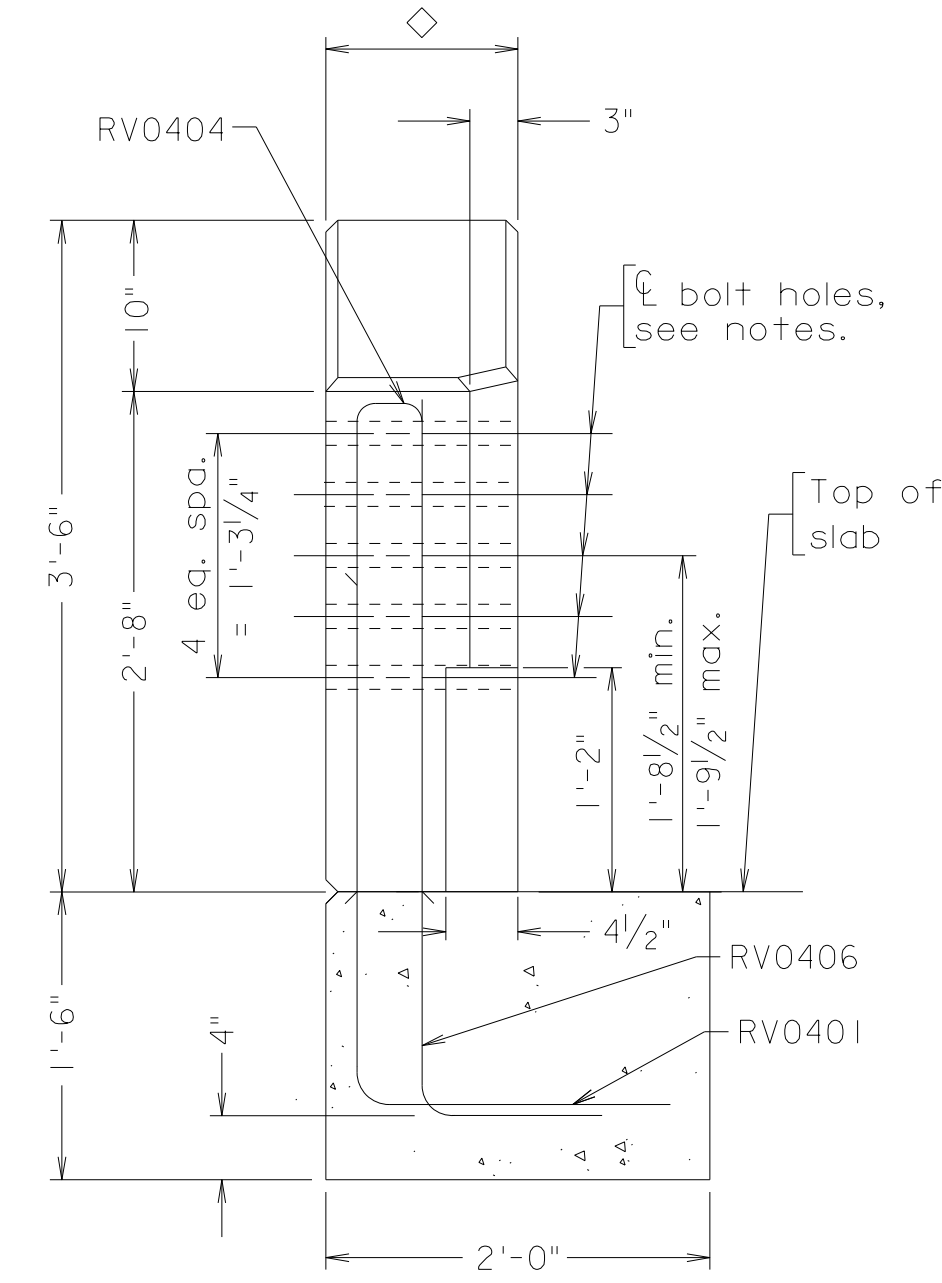
Terminal walls are detailed to take guardrail attachment for MGS.

Holes, where shown, shall be formed with sleeves of 1/4" diameter nominal pipe.

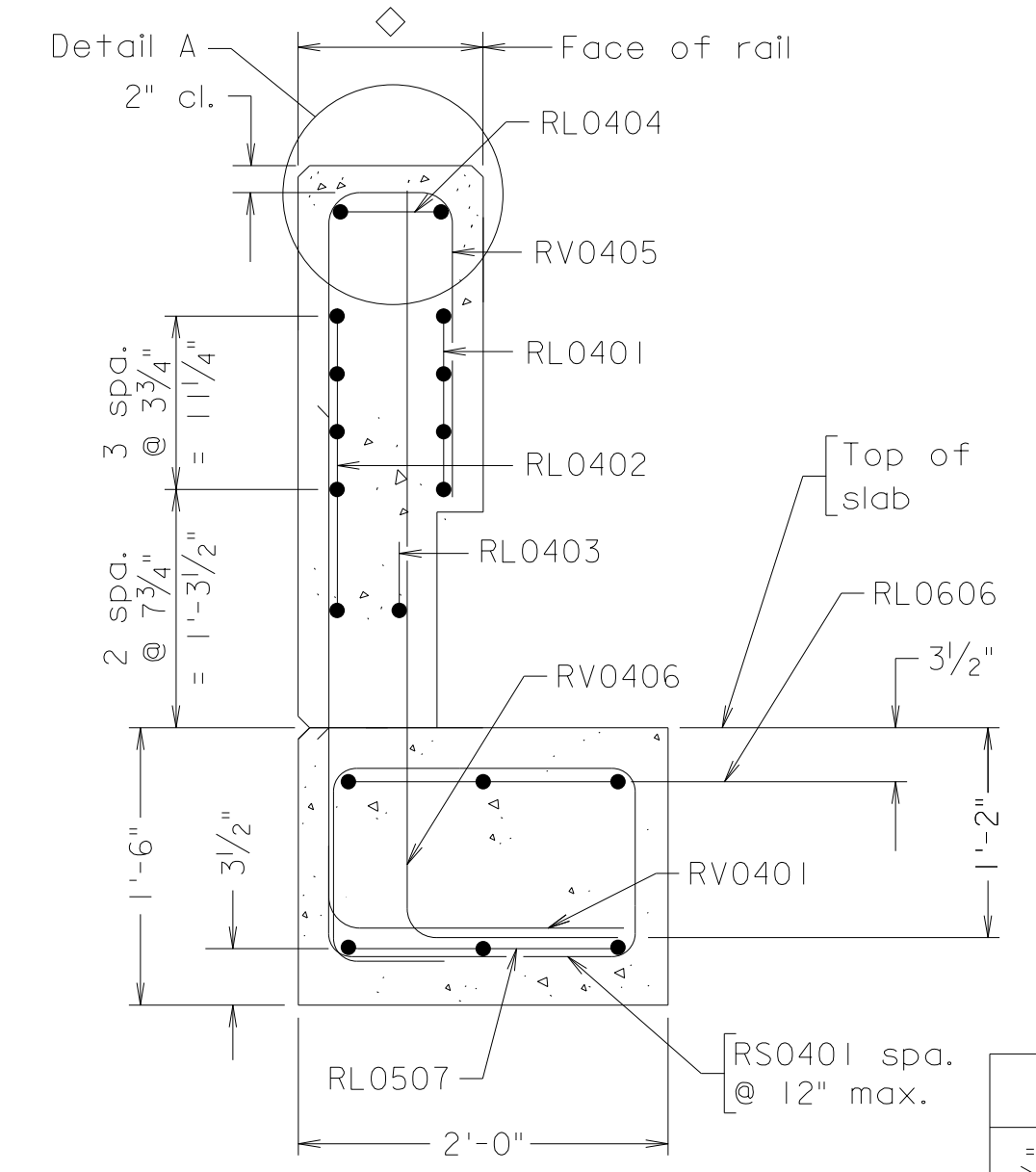
Bid price for architectural treatment includes concrete in relief and coping.



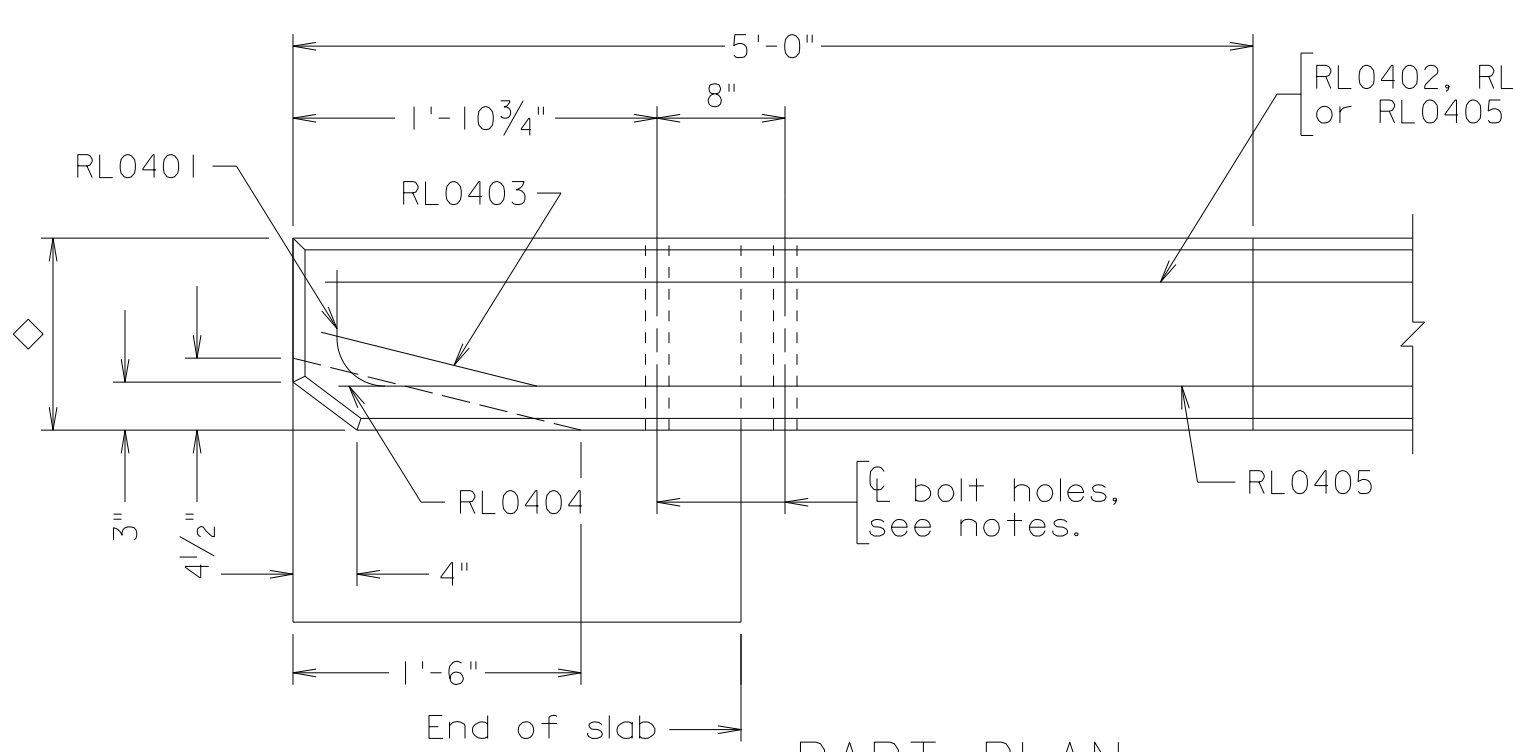
FULL INTEGRAL OR SEMI-INTTEGRAL ABUTMENT



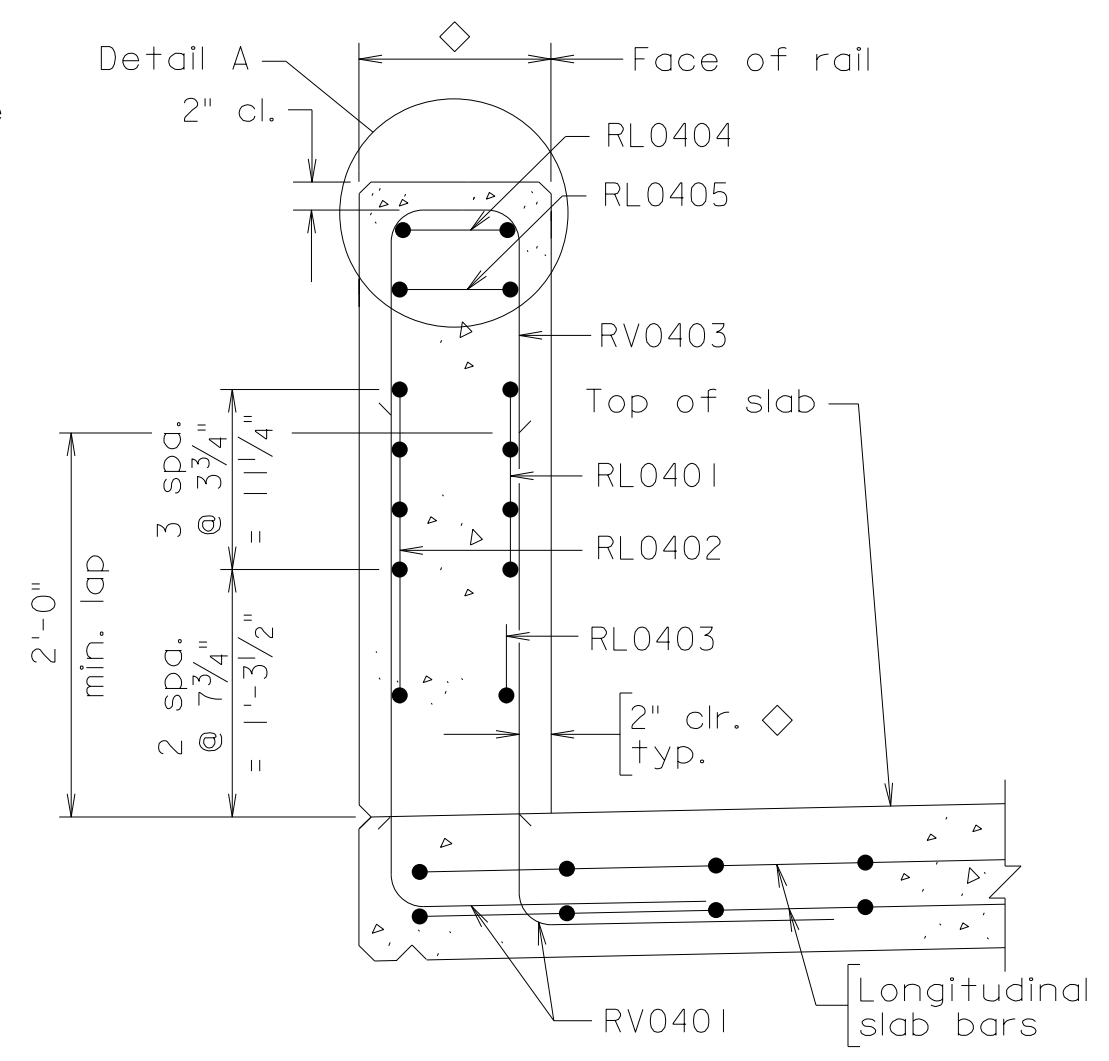
VIEW A-A  
Showing RV04 bars only for clarity



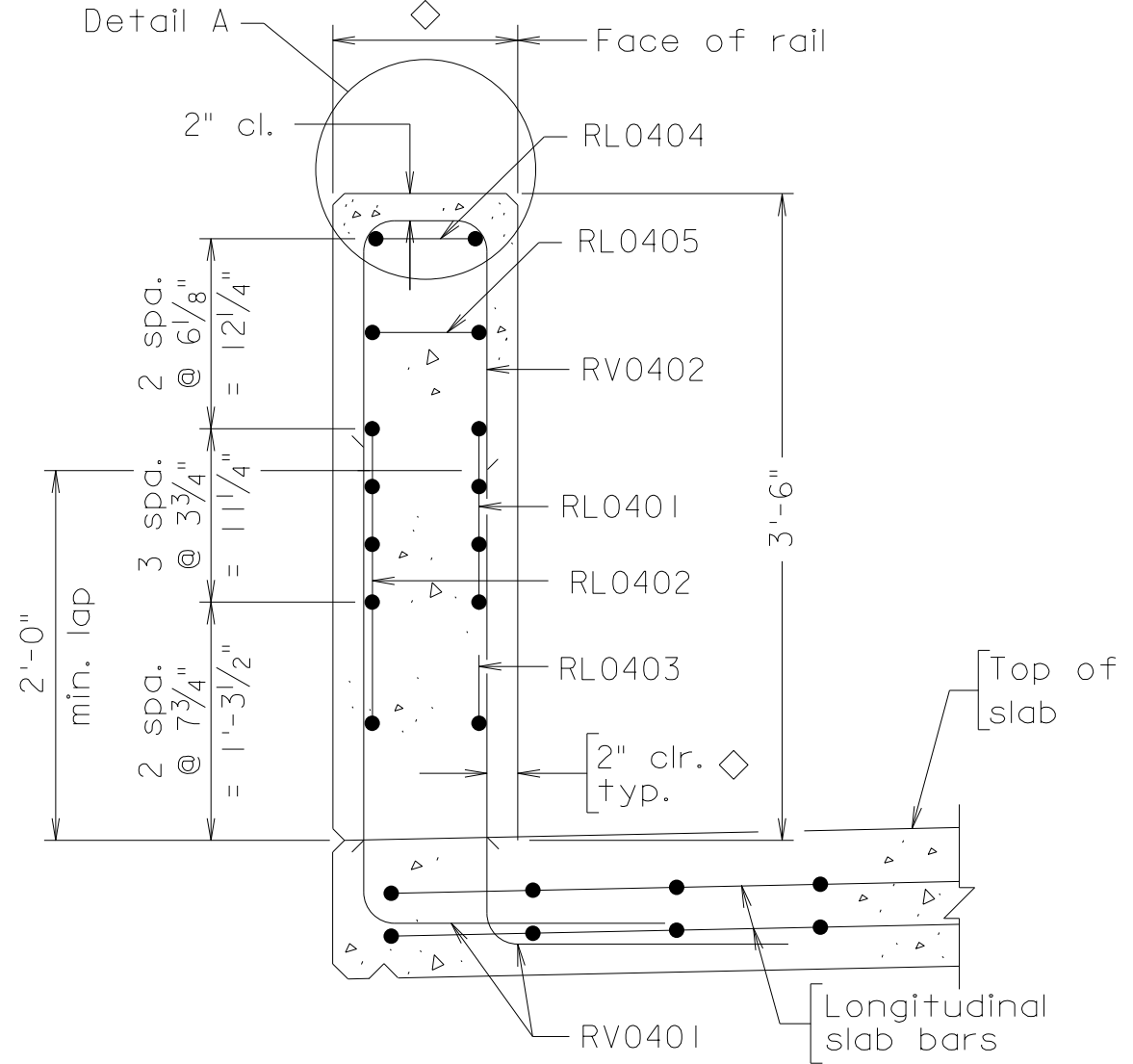
SECTION B-B



PART PLAN  
RV04 series bars not shown for clarity



SECTION C-C  
Transverse slab bar not shown for clarity



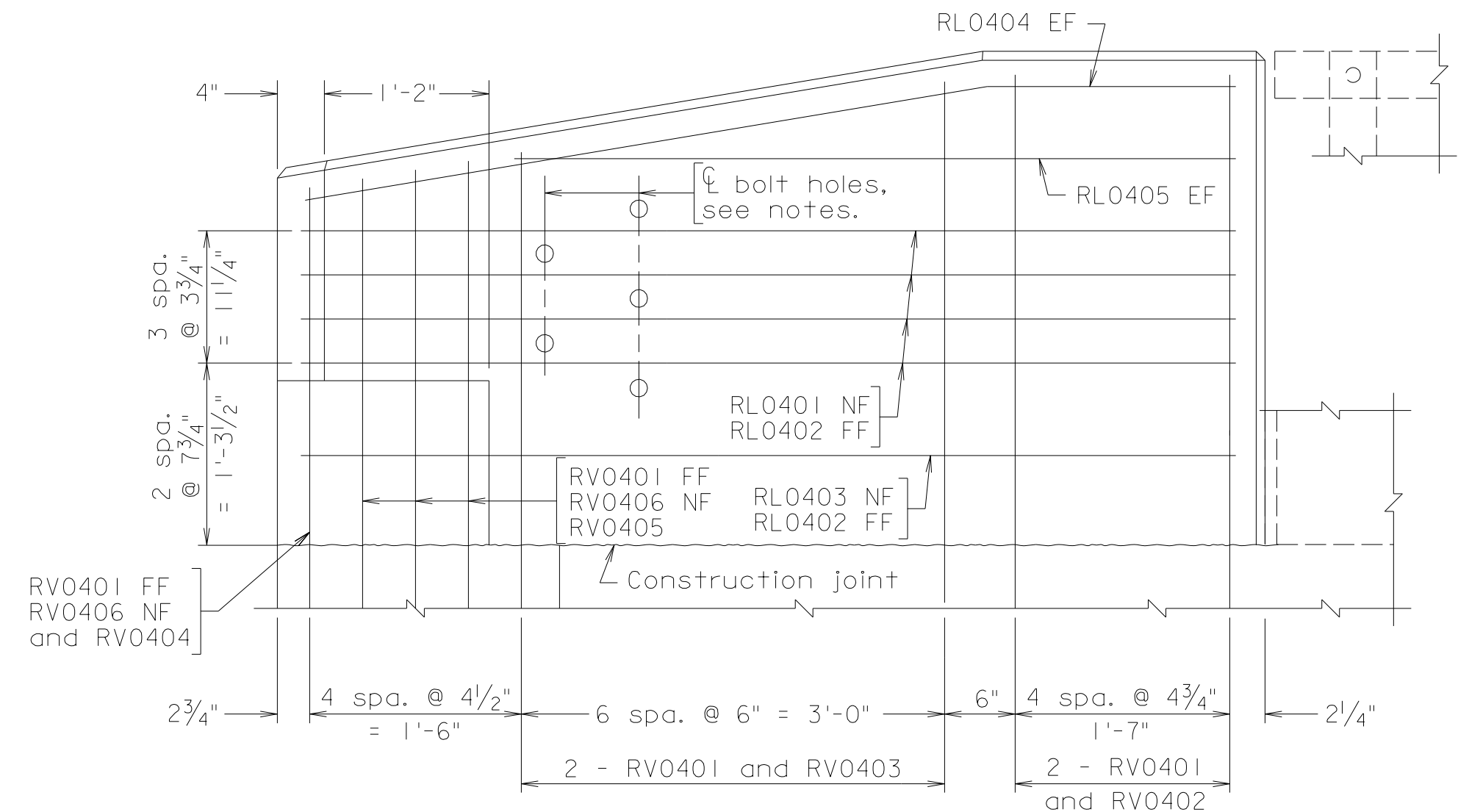
SECTION D-D

Transverse bars in deck slab and RL04 series extended from railing not shown for clarity

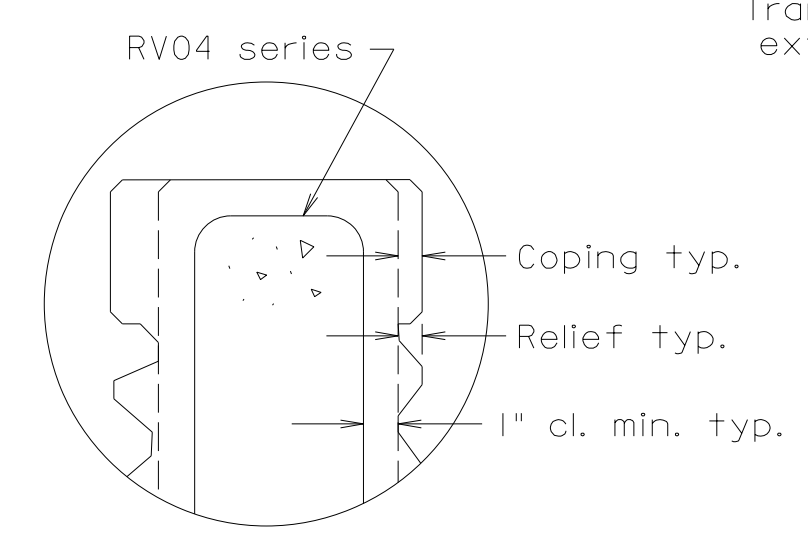
Mark	Size	No.	Pin ø	Length	Location
RL0401	#4	—	6"	7'-2"	Terminal wall
RL0402	#4	—	—	6'-8"	Terminal wall
RL0403	#4	3"	3"	6'-8 1/2"	Terminal wall
RL0404	#4	—	3"	6'-9"	Terminal wall
RL0405	#4	—	—	5'-2"	Terminal wall
RL0606	#6	4 1/2"	—	5'-9 1/2"	Terminal wall end support
RS0401	#4	3"	—	6'-2"	Terminal wall end support
RV0401	#4	3"	3"	4'-4"	Terminal wall
RV0402	#4	3"	—	7'-1 1/2"	Terminal wall
RV0403	#4	3"	—	From 6'-0 1/2" to 7'-0 1/2"	Terminal wall
RV0404	#4	3"	—	5'-2"	Terminal wall
RV0405	#4	3"	—	From 4'-3 1/2" to 4'-6 1/2"	Terminal wall
RV0406	#4	3"	—	From 4'-7 3/4" to 4'-10"	Terminal wall

Mark	Size	No.	Pin ø	Length	Location
RV0402	#4	8"	—	3'-4"	
RV0403	#4	8"	—	vary from 2'-9/2" to 3'-3 1/2" by 1"	
RV0404	#4	4"	—	2'-6 1/4"	
RV0405	#4	8"	—	vary from 2'-7" to 2'-8 1/2" by 3/4"	
RS0401	#4	11"	—	1'-3" + 0" by 1'-4 1/2" by 3/4"	

No.	Description	Date	Designed: S&B DIV	Drawn: S&B DIV	Checked: S&B DIV	Date	Plan No.	Sheet No.
						January 2026		24 of 30



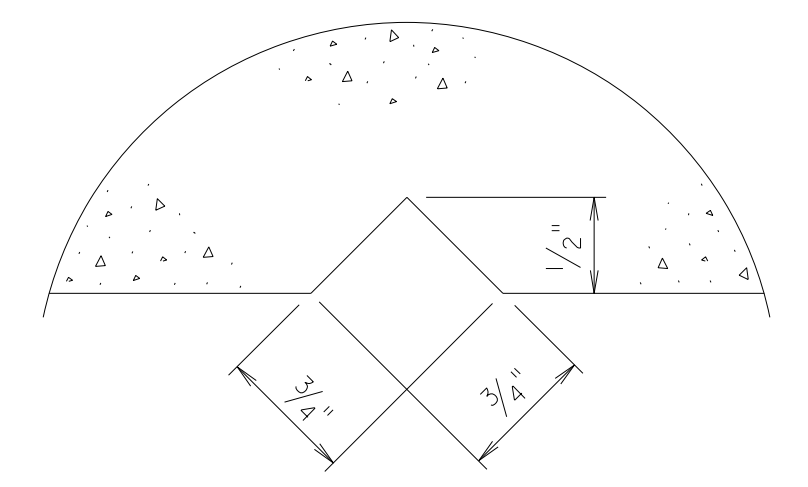
TERMINAL WALL - ELEVATION



DETAIL A  
Not to scale

Shown with architectural treatment on both sides

For dimensions and architectural treatment details, see sheet XX. For minimum cover, see Detail A.



SECTION E-E  
Full scale

Groove detail for both sides of rail

90% PLANS  
THESE PLANS NOT TO BE USED FOR CONSTRUCTION

Scale: 1" = 1'-0" unless otherwise noted.

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Sealed and Signed by:  
Junyi Meng  
Lic. No. 033572  
On the date of  
April 30, 2024

A copy of the original sealed and signed standard drawing is on file in the Central Office.

KIMLEY-HORN & ASSOC.  
RICHMOND, VA  
STRUCTURAL ENGINEER

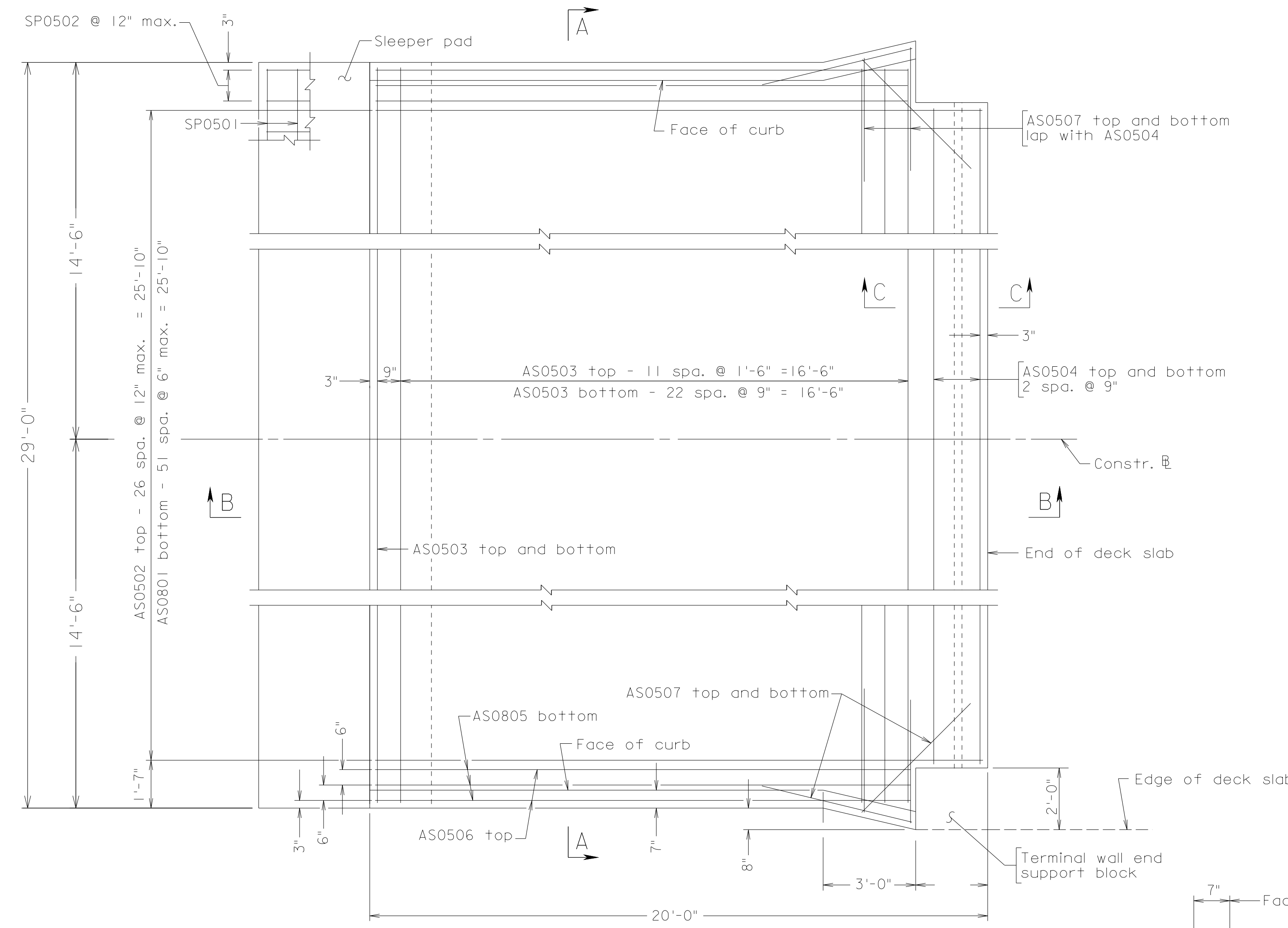
STATE	FEDERAL AID	STATE	SHEET NO.
VA.	PROJECT STP-5104 (326)	ROUTE U000-104-365, B620, C501, P101	---

Notes:

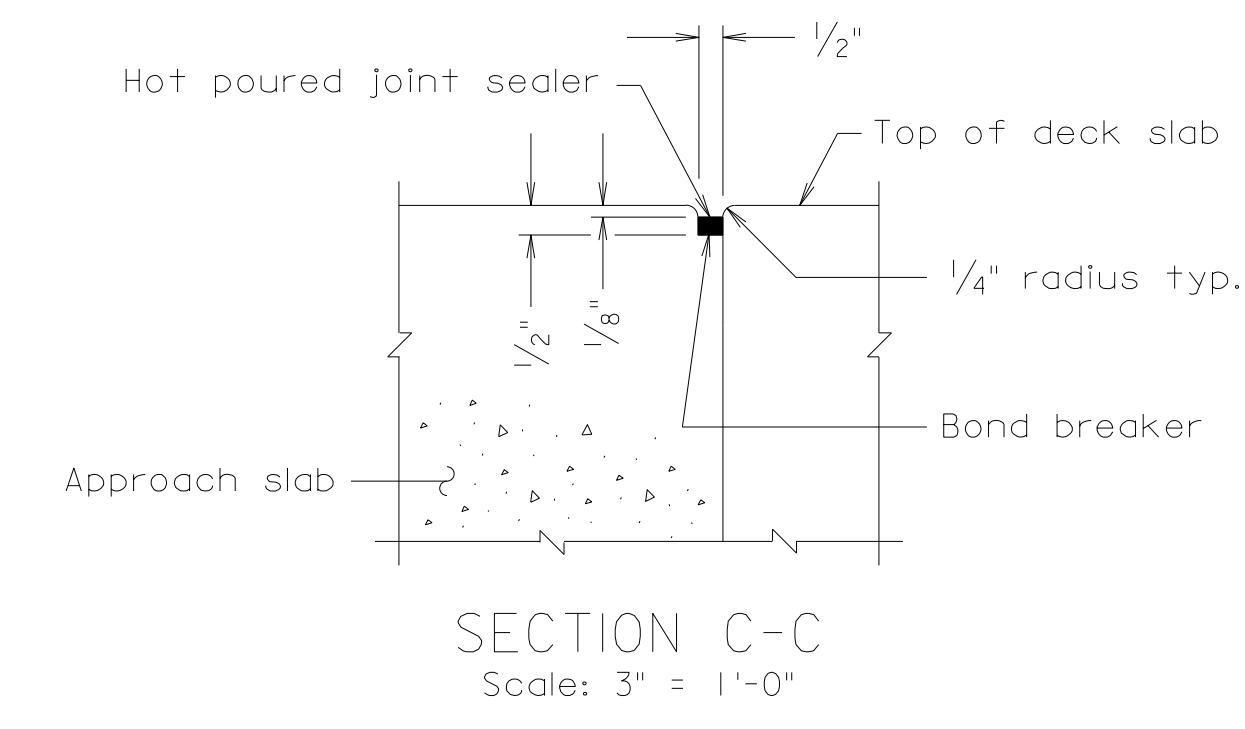
If material under approach slab will be exposed for more than two weeks, prime the material with 0.35 gal per sq. yd. liquid asphalt material type RC-70, RC-250 or MC-250. Cost included in select backfill (see sheet...).

Approach slab shall be grooved in accordance with Section 404.07(f) of the Specifications. Payment for grooving is included in bridge deck grooving quantities.

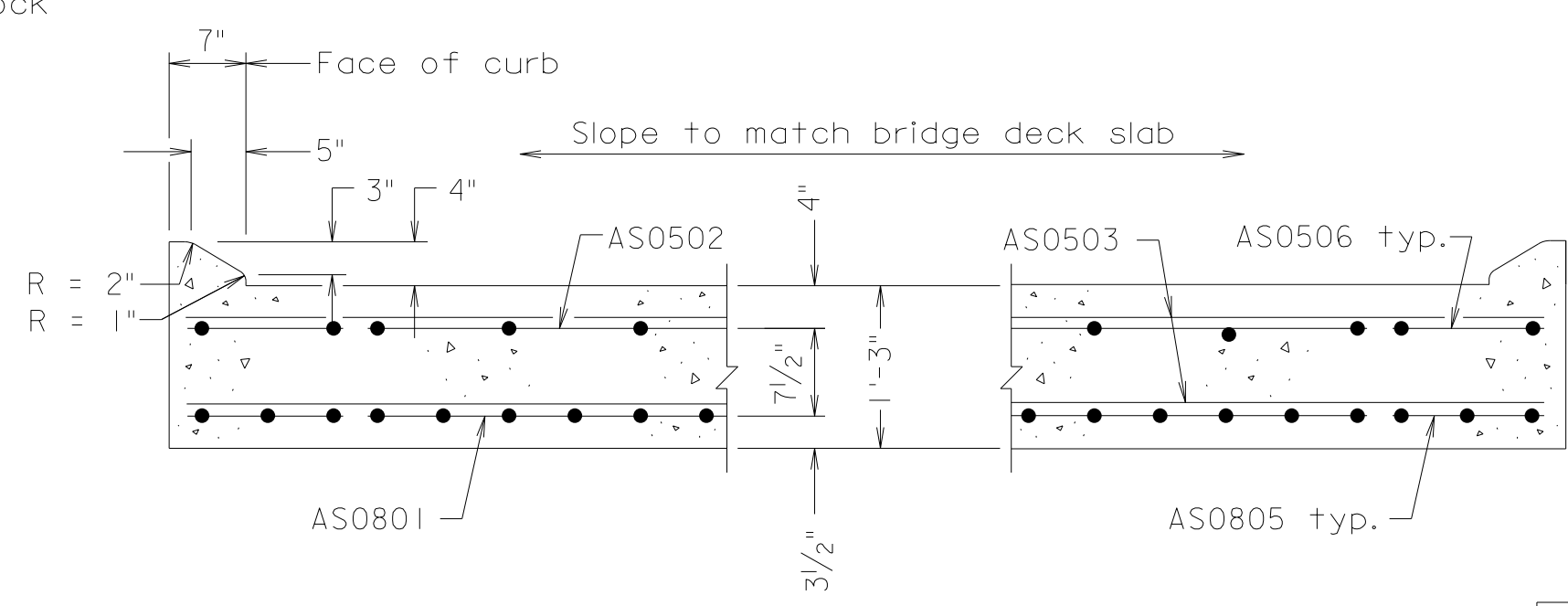
Payment for the CD-2 and the asphalt on top of sleeper pad is included in the road plans.



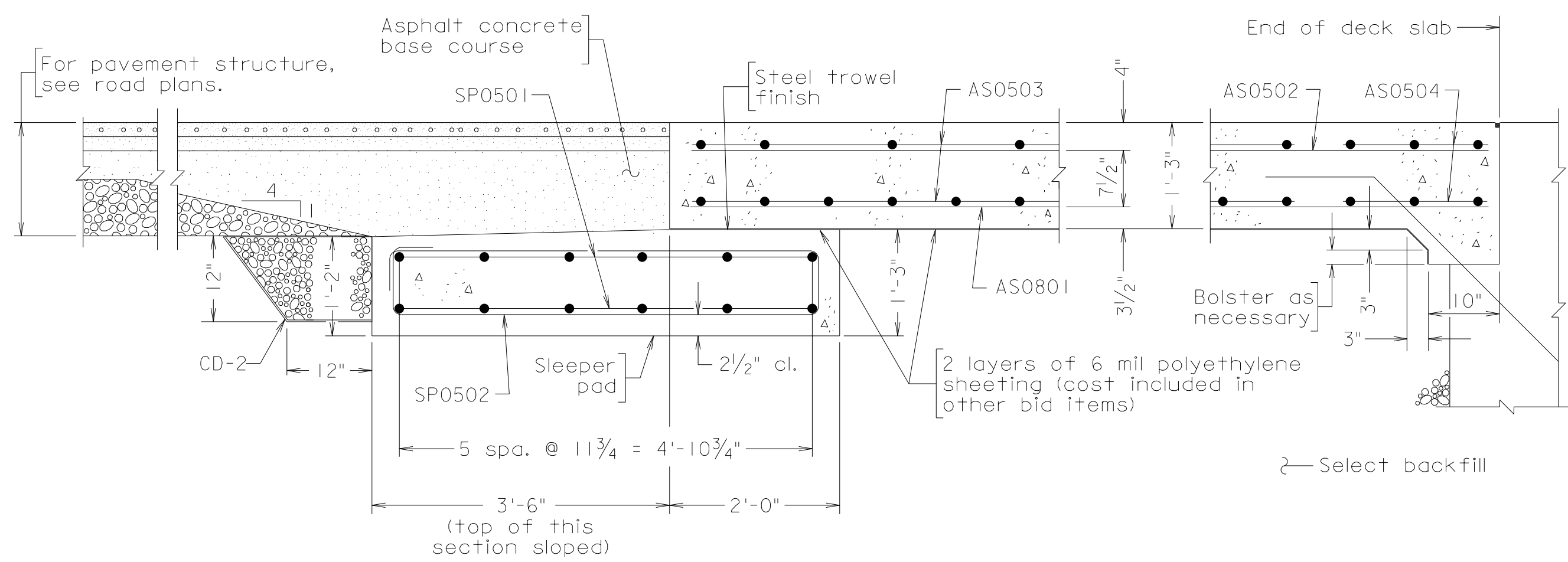
PLAN  
Details typical on each side of Constr. E



SECTION C-C  
Scale: 3" = 1'-0"



SECTION A-A  
Curb details typical  
Scale: 3/4" = 1'-0"



SECTION B-B  
Scale: 3/4" = 1'-0"

REINFORCING STEEL SCHEDULE						
SP0502						
Mark	No.	Size	Pin $\phi$	Length	Location	
AS0801	52	#8	---	19'-8"	Bottom longitudinal	
AS0502	27	#5	---	19'-8"	Top longitudinal	
AS0503	37	#5	---	28'-8"	Top and bottom transverse	
AS0504	6	#5	---	25'-8"	Top and bottom transverse	
AS0805	6	#8	---	17'-5"	Bottom longitudinal	
AS0506	4	#5	---	17'-5"	Top longitudinal	
AS0507	16	#5	---	5'-0"	Near terminal wall end block	
SP0501	12	#5	---	28'-8"	Sleeper pad	
SP0502	30	#5	2 1/2"	12'-2"	Sleeper pad	

Dimensions in bending diagram are out-to-out of bars.  
Number of bars shown is for one approach slab and sleeper pad.

ESTIMATED QUANTITIES			
		Concrete Class A4 Bridge Approach Slab ⊗ CY	Corrosion Resistant Reinforcing Steel Class 1, Grade 60 min. ⊗ LB
Abutment A	Approach slab	27.8	4990
	Sleeper pad	7.2	740
Abutment B	Approach slab	27.8	4990
	Sleeper pad	7.2	740
Totals		70.0	11460

⊗ Denotes items to be paid for on basis of plan quantities in accordance with Road and Bridge Specifications.

CITY OF CHARLOTTESVILLE					
APPROACH SLABS BAS-AG-1					
No.	Description	Date	Designed: S&B DIV	Date	Plan No.
	Revisions		Drawn: S&B DIV	January 2026	
			Checked: S&B DIV		Sheet No.
					30 of 30

Sealed and Signed by:  
Junyi Meng  
Lic. No. 033572  
On the date of  
April 26, 2023

A copy of the original  
sealed and signed  
drawing is on file in the  
Central Office.

KIMLEY-HORN & ASSOC.  
RICHMOND, VA  
STRUCTURAL ENGINEER